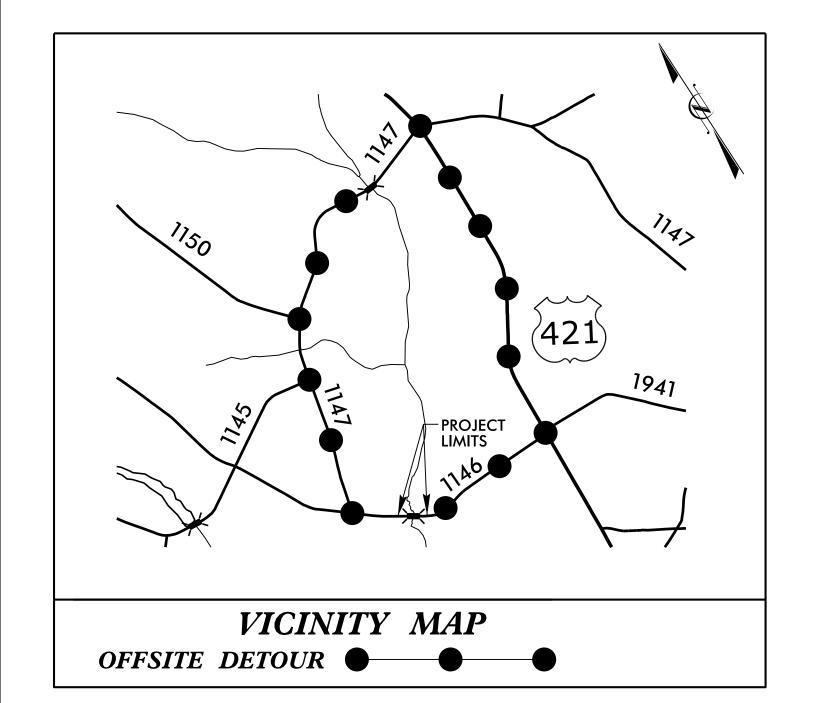
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This file or an individual page shall not be considered a certified document.

See Sheet 1-A For Index of Sheets

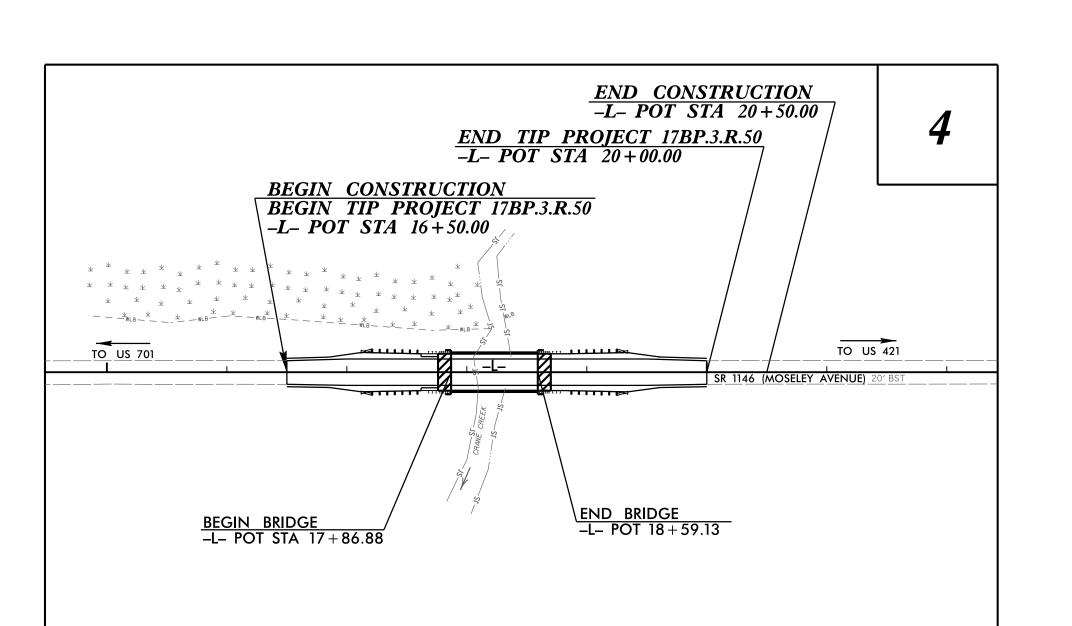


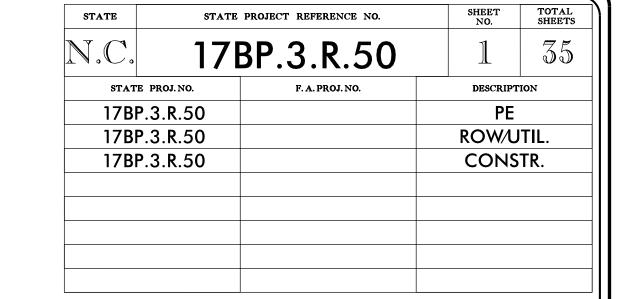
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

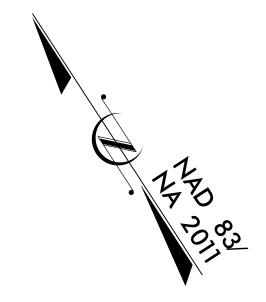
SAMPSON COUNTY

LOCATION: REPLACE BRIDGE NO. 17 OVER CRANE CREEK ON SR 1146 (MOSELEY AVENUE)

TYPE OF WORK: GRADING, DRAINAGE, PAVING AND STRUCTURE







DOCUMENT NOT CONSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED**

PLANS 50 25 0 PROFILE (HORIZONTAL)

PROFILE (VERTICAL)

GRAPHIC SCALES

DESIGN DATA

ADT 2013 = 330ADT 2033 = 660K = 10 %D = 60 %V = 60 MPH* TTST = 2% DUAL 4% FUNC CLASS = LOCAL

SUBREGIONAL TIER

PROJECT LENGTH

LENGTH OF ROADWAY PROJECT 17BP.3.R.50 = 0.052 MILES LENGTH OF STRUCTURE PROJECT 17BP.3.R.50 = 0.014 MILES

TOTAL LENGTH OF PROJECT 17BP.3.R.50 = 0.066 MILES

Prepared in the Office of: HNTB NORTH CAROLINA, P.C. 343 E. Six Forks Road, Suite 200 Raleigh, North Carolina 27609 NC License No: C-1554

2012 STANDARD SPECIFICATIONS

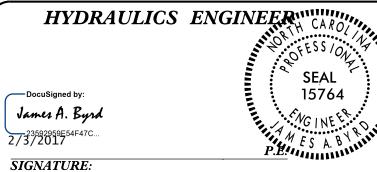
RIGHT OF WAY DATE: **NOVEMBER 23, 2016**

LETTING DATE: MARCH 16, 2017

DAVID W. BASS, PE PROJECT ENGINEER

MONICA J. DUVAL PROJECT DESIGN ENGINEER

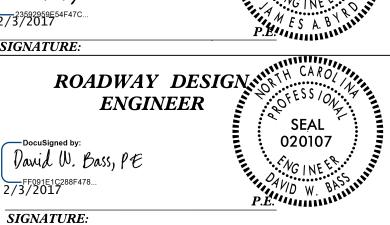
ALTON R. EDGERTON

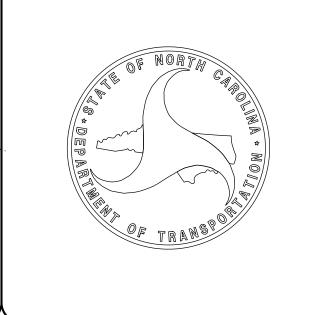


2/3/2017

SIGNATURE:







INDEX OF SHEETS

SHEET NUMBER <u>SHEET</u> TITLE SHEET INDEX OF SHEETS, GENERAL NOTES & LIST OF STANDARDS 1A-1 1B₋₁ SYMBOLOGY SHEET 1C SURVEY CONTROL SHEET 2A-1 TYPICAL SECTION SHEET 2C-1 STRUCTURE ANCHOR UNIT DETAIL 2C-2 METHOD OF CLEARING - MODIFIED METHOD III 3B-1 EARTHWORK, PAVEMENT REMOVAL, GUARDRAIL SUMMARY, ROW SUMMARY, & DRAINAGE SUMMARY SHEET PLAN & PROFILE SHEET TMP-1 THRU TMP-2 TRAFFIC CONTROL PLANS

EROSION CONTROL PLANS

CROSS SECTION SHEETS

STRUCTURE PLANS

GENERAL NOTES: 2012 SPECIFICATIONS EFFECTIVE: 01–17–2012

REVISED: 10–31–2014

GRADE LINE: GRADING AND SURFACING:

> THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY MODIFIED METHOD III.

SUPERELEVATION:

EC-1 THRU EC-4

X-1 THRU X-3

S-1 THRU S-15

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

SIDE ROADS:

THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

SUBSURFACE PLANS:

NO SUBSURFACE PLANS ARE AVAILABLE ON THIS PROJECT. THE CONTRACTOR SHOULD MAKE HIS OWN INVESTIGATION AS TO THE SUBSURFACE CONDITIONS.

END BENTS:

THE ENGINEER SHALL CHECK THE TRUCTURE END BENT PLANS, DETAILS, AND CROSS SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROCHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE

WATER – SAMPSON COUNTY WATER

PHONE – CENTURYLINK

POWER - FOUR COUNTY EMC

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

EFF. 01–17–2012 REV. 02-29-2016

2012 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch – N. C. Department of Transportation – Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO. DIVISION 2 – EARTHWORK

Method of Clearing – Modified Method III (Use detail in lieu of standard)

Guide for Grading Subgrade — Secondary and Local

Method of Obtaining Superelevation – Two Lane Pavement

DIVISION 3 – PIPE CULVERTS 300.01 Method of Pipe Installation

DIVISION 4 – MAJOR STRUCTURES

422.10 Reinforced Bridge Approach Fills

560.01 Method of Shoulder Construction – High Side of Superelevated Curve – Method I

DIVISION 8 – INCIDENTALS

Concrete Base Pad for Drainage Structures Frames and Narrow Slot Flat Grates

840.29

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

840.35 Traffic Bearing Grated Drop Inlet – for Cast Iron Double Frame and Grates

Drainage Structure steps 840.66

Concrete Curb, Gutter and Curb & Gutter 846.01

862.01 **Guardrail Placement** 862.02 Guardrail Installation

Structure Anchor Units (Beg. March 2013 letting use detail in lieu of standard)

876.01 Rip Rap in Channels

Guide for Rip Rap at Pipe Outlets 876.02

1A-1 **ROADWAY DESIGN ENGINEER** SEAL 020107

SHEET NO.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO.

17BP.3.R.50

State Line

County Line -

City Line

Township Line

Reservation Line —

PROJECT REFERENCE NO. 17BP.3.R.50

BOUNDARIES AND PROPERTY:

Property Line	
Existing Iron Pin	<u></u>
Property Corner	
Property Monument	
Parcel/Sequence Number	
Existing Fence Line	××
Proposed Woven Wire Fence	
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary	
Proposed Wetland Boundary	
Existing Endangered Animal Boundary —	EAB
Existing Endangered Plant Boundary —	EPB
Existing Historic Property Boundary	——— НРВ ————
Known Contamination Area: Soil	
Potential Contamination Area: Soil	
Known Contamination Area: Water	
Potential Contamination Area: Water	
Contaminated Site: Known or Potential —	— ** ** **
BUILDINGS AND OTHER CU	LTURE:
Gas Pump Vent or U/G Tank Cap	
Gas Pump Vent or U/G Tank Cap ———————————————————————————————————	
Sign —	<u>©</u> S
Sign ————————————————————————————————————	⊙ s O W ✓ ✓
Sign Well Small Mine	
Sign Well Small Mine Foundation Area Outline	
Sign Well Small Mine Foundation Area Outline Cemetery	
Sign Well Small Mine Foundation Area Outline	
Sign Well Small Mine Foundation Area Outline Cemetery Building	
Sign Well Small Mine Foundation Area Outline Cemetery Building School	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY:	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow	□ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring	

CONVENTIONAL PLAN SHEET SYMBOLS

Note: Not to Scale

*S.U.E. = Subsurface Utility Engineering

Bridge Wing Wall, Head Wall and End Wall

RAILROADS:	
Standard Gauge ————	CSX TRANSPORTATION
RR Signal Milepost	⊙ MILEPOST 35
Switch ————	SWITCH
RR Abandoned	
RR Dismantled	
RIGHT OF WAY:	
Baseline Control Point	•
Existing Right of Way Marker	\triangle
Existing Right of Way Line	
Proposed Right of Way Line	$\frac{R}{W}$
Proposed Right of Way Line with Iron Pin and Cap Marker	
Proposed Right of Way Line with Concrete or Granite R/W Marker	R W
Proposed Control of Access Line with Concrete C/A Marker	
Existing Control of Access	
Proposed Control of Access —	<u> </u>
Existing Easement Line ————————————————————————————————————	——Е——
Proposed Temporary Construction Easement –	——Е——
Proposed Temporary Drainage Easement —	TDE
Proposed Permanent Drainage Easement —	PDE
Proposed Permanent Drainage / Utility Easement	DUE
Proposed Permanent Utility Easement ———	PUE
Proposed Temporary Utility Easement ———	TUE
Proposed Aerial Utility Easement ————	AUE——
Proposed Permanent Easement with Iron Pin and Cap Marker	♦
ROADS AND RELATED FEATURES	S:
Existing Edge of Pavement	
Existing Curb	
Proposed Slope Stakes Cut	<u>C</u>
Proposed Slope Stakes Fill	<u>F</u>
Proposed Curb Ramp	
Existing Metal Guardrail	
Proposed Guardrail ————	<u> </u>
Existing Cable Guiderail	
Proposed Cable Guiderail	
Equality Symbol	lacktriangle
Pavement Removal ————	
VEGETATION:	
Single Tree	₩
Single Shrub	₿
Hedge ————	
Woods Line	<u>(;,(;,(;,(;,</u>

Orchard Vineyard **EXISTING STRUCTURES: MAJOR:** Bridge, Tunnel or Box Culvert —

MINOR: Head and End Wall Pipe Culvert Footbridge -Drainage Box: Catch Basin, DI or JB Paved Ditch Gutter

UTILITIES:

Storm Sewer

Storm Sewer Manhole

•
6
-
-6-
P
otag
•—•
— — — P — — -
——————————————————————————————————————
P

TELEPHONE:	
Existing Telephone Pole	-
Proposed Telephone Pole ————	-0-
Telephone Manhole	\bigcirc
Telephone Pedestal —————	
Telephone Cell Tower	, ————————————————————————————————————
U/G Telephone Cable Hand Hole ————	H_{H}
U/G Telephone Cable LOS B (S.U.E.*) — – –	t
U/G Telephone Cable LOS C (S.U.E.*) ————	T
U/G Telephone Cable LOS D (S.U.E.*) ————	Т
U/G Telephone Conduit LOS B (S.U.E.*) —	TC
U/G Telephone Conduit LOS C (S.U.E.*) —	— — тс— — —
U/G Telephone Conduit LOS D (S.U.E.*)———	TC
U/G Fiber Optics Cable LOS B (S.U.E.*) — – –	— — т ғо— — ·
U/G Fiber Optics Cable LOS C (S.U.E.*)———	— — T FO— — —
U/G Fiber Optics Cable LOS D (S.U.E.*)———	T FO

WATER:

] CONC WW [

WAIER:	
Water Manhole ————————————————————————————————————	W
Water Meter —	
Water Valve	\otimes
Water Hydrant —	♦
U/G Water Line LOS B (S.U.E*)	
U/G Water Line LOS C (S.U.E*)	w
U/G Water Line LOS D (S.U.E*)	
Above Ground Water Line	A/G Water
ΓV:	
TV Pedestal ————————————————————————————————————	C
TV Tower —	\bigotimes
U/G TV Cable Hand Hole —	H _H
U/G TV Cable LOS B (S.U.E.*)	TV

U/G TV Cable LOS C (S.U.E.*) ------

U/G Fiber Optic Cable LOS C (S.U.E.*) — — — — TV FO — —

U/G Fiber Optic Cable LOS D (S.U.E.*) — TV FO TV FO

U/G TV Cable LOS D (S.U.E.*)

GAS:

as Valve	\Diamond
as Meter ———————————————————————————————————	\Diamond
G Gas Line LOS B (S.U.E.*)	
G Gas Line LOS C (S.U.E.*)	
G Gas Line LOS D (S.U.E.*)	G
bove Ground Gas Line	A/G Gas
CONC CICCING COC CITY	

SANITARY SEWER:

Sanitary Sewer Manhole —

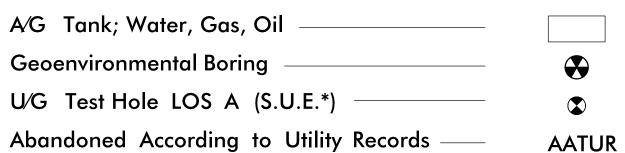
Sanitary Sewer Cleanout

U/G Sanitary Sewer Line ——————	ss
Above Ground Sanitary Sewer ————	A/G Sanitary Sewer
SS Forced Main Line LOS B (S.U.E.*) ———	— — — FSS — — — –
SS Forced Main Line LOS C (S.U.E.*) ———	——————————————————————————————————————
SS Forced Main Line LOS D (S.U.E.*)———	FSS

MISCELLANEOUS:

End of Information

Utility Pole	•
Utility Pole with Base ————————————————————————————————————	
Utility Located Object —	\odot
Utility Traffic Signal Box —	S
Utility Unknown U/G Line LOS B (S.U.E.*)	
U/G Tank; Water, Gas, Oil —————	
Underground Storage Tank, Approx. Loc. ——	UST



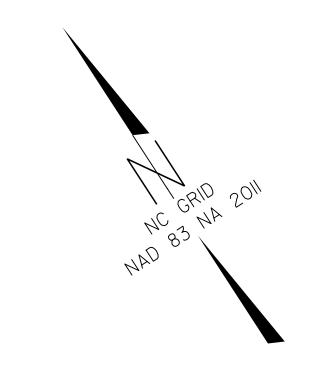
E.O.I.

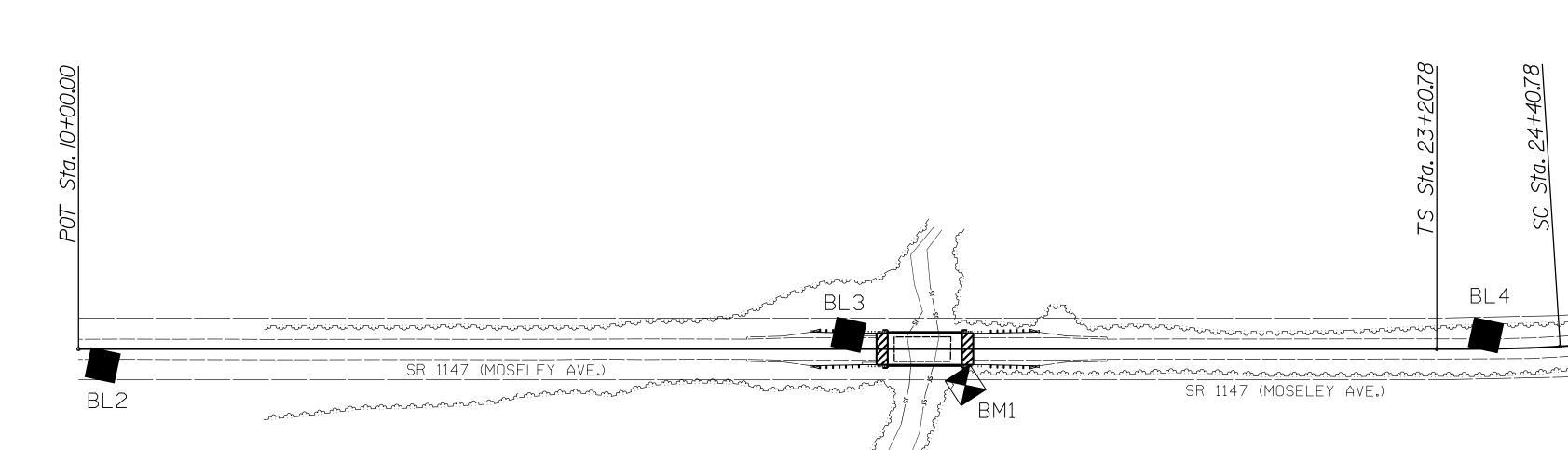
SURVEY CONTROL SHEET 81-0017

PROJECT REFERENCE NO. SHEET NO.

WBS# 17BP.3.R.50 1C

Location and Surveys





PDE					
ALIGN	STATION	OFFSET	NORTH	EAST	
L	17+50.00	50.00	412598.78521	2215438.50425	
L	17+50.00	30.00	412615.55471	2215449.40305	
L	17+75.00	30.00	412601.93121	2215470.36493	
L	17+75.00	50.00	412585.16171	2215459.46613	
L	18+30.00	30.00	412571.95950	2215516.48105	
L	18+30.00	38.00	412565.25170	2215512.12153	
L	18+50.00	-30.00	412611.36920	2215565.94696	
L	18+50.00	-45.00	412623.94633	2215574.12106	
L	19+00.00	-45.00	412596.69932	2215616.04481	
L	19+00.00	-30.00	412584.12220	2215607.87071	
L	19+15.00	38.00	412518.93180	2215583.39191	
L	19+15.00	30.00	412525.63960	2215587.75143	

		L	
TYPE	STATION	NORTH	EAST
POT	10+00.00	413049.4140	2214836.8950
TS	23+20.78	412329.6690	2215944.3346
SC	24+40.78	412266.4314	2216Ø46.2946
CS	30+73.99	412156.5263	2216657.93Ø4
ST	31+93.99	412180.3116	2216775.5276

412209.5180

BOINT RF	DESC.	NORTH	EAST	ELEVATION	L STATION	OFFSET
GPS1	GPS CAP & REBAR	414620.1450	2212811.3540	115.09	OUTSIDE PROJECT	 Γ LIMITS
GPS2	GPS CAP & REBAR	413966.0260	2213637.3360	104.29	OUTSIDE PROJECT	T LIMITS
BL1	TRV CAP & REBAR	413344.4180	2214375.1320	89.21	OUTSIDE PROJECT	T LIMITS
BL2	TRV CAP & REBAR	413023.2660	2214847.8460	79.25	10+23.43	15.96 RT
BL3	TRV CAP & REBAR	412652.8450	2215472.1060	71.76	17+48.71	13.64 LT
BL4	TRV CAP & REBAR	412314.8140	2215991.5040	95.63	23+68.56	13.Ø9 LT

ELEVATION = 74.13

E 2215541

L STATION 18+62.00 35 RIGHT

N 41255Ø

RR_SPIKE_2Ø"_GUM

NOTES:

○ INDICATES GEODETIC CONTROL MONUMENTS USED OR SET FOR HORIZONTAL AND VERTICAL PROJECT CONTROL BY THE NCDOT LOCATION AND SURVEYS UNIT.

PROJECT CONTROL ESTABLISHED USING GNSS (GLOBAL NAVIGATION SATELLITE SYSTEM).

22169Ø5.688Ø

THE CONTROL DATA FOR THIS PROJECT CAN BE FOUND ELECTRONICALLY BY SELECTING PROJECT CONTROL DATA AT:

HTTPS://CONNECT.NCDOT.GOV/RESOURCES/LOCATION/

THE FILES TO BE FOUND ARE AS FOLLOWS: 81-0017_LS_CONTROL.TXT

33+27.39

SITE CALIBRATION INFORMATION HAS NOT BEEN PROVIDED FOR THIS PROJECT. IF FURTHER INFORMATION IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

DATUM DESCRIPTION

THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCDOT FOR MONUMENT "GPS-2"

WITH NAD 83/NSRS 2007 STATE PLANE GRID COORDINATES OF NORTHING: 413966.026(ft) EASTING: 2213637.336(ft) ELEVATION: (ft)

THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 0.9998934300

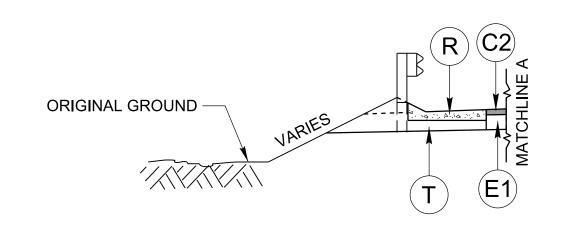
THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM "GPS-2" TO -L- L STATION 10+00.00 IS S 52° 36′ 56″ E 1,509.68′

ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES VERTICAL DATUM USED IS NAVD 88

NOTE: DRAWING NOT TO SCALE

	PAVEMENT SCHEDULE	
C1	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YARD IN EACH OF TWO LAYERS.	
C2	PROP. VARIABLE DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A AT AN AVERAGE RATE OF 110 LBS. PER SQ. YARD PER INCH. DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 1" IN DEPTH OR GREATER THAN 1.5" IN DEPTH.	
E1	PROP. APPROX. 5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 570 LBS. PER SQ. YARD.	
E2	PROP. VARIABLE DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0B AT AN AVERAGE RATE OF 114 LBS. PER SQ. YARD PER INCH. DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5.5" IN DEPTH.	
R	SHOULDER BERM GUTTER	
Т	EARTH MATERIAL	

ALL PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE

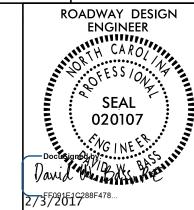


DETAIL A SHOULDER BERM GUTTER LOCATIONS

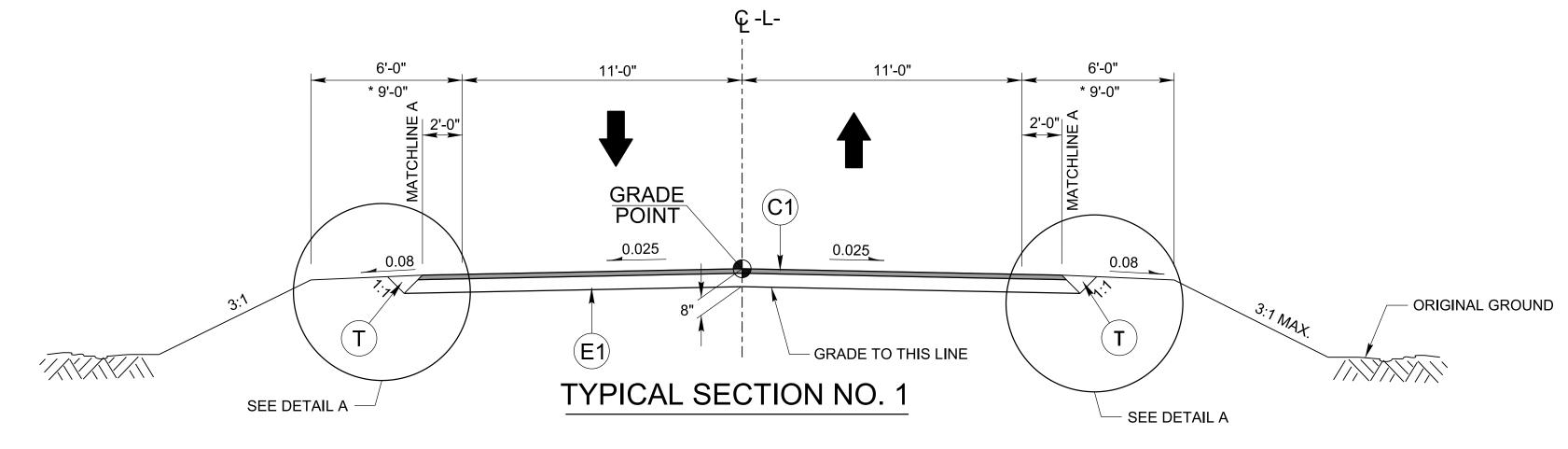
-L- STA 17+61.88 to STA 17+76.00 LT/RT

HNTB NORTH CAROLINA, P.C.
343 E. Six Forks Road, Suite 200
Raleigh, North Carolina 27609
NC License No: C-1554

PROJECT REFERENCE NO. SHEET NO. 2A-1 17BP.3.R.50

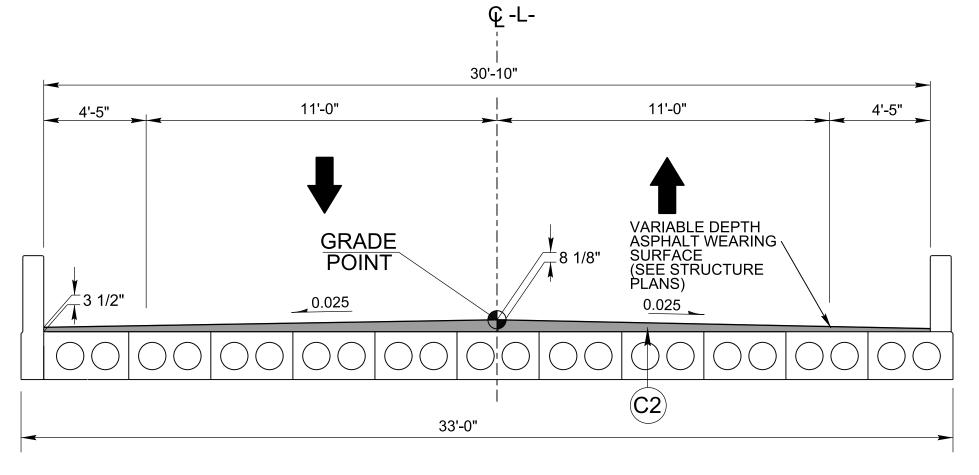


DOCUMENT NOT CONSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED**



USE TYPICAL SECTION NO. 1 FROM:

-L- STA 16+50.00 TO STA 17+86.88(BRIDGE) -L- STA 18+59.13(BRIDGE) TO STA 20+00.00



USE TYPICAL SECTION NO. 2 FROM: -L- STA 17+86.88 TO STA 18+59.13

TYPICAL SECTION NO. 2 CORED SLAB BRIDGE OVERLAY

PROJECT REFERENCE NO. SHEET NO. 17BP.3.R.50 2C-1

NORTH CAROLINA DEPT, OF TRANSPORTATION SYAWHOIH OF HIGHWAYS .D.N , HDIBLAR 862d03 862d03 RAIL ON BRIDGE - SUB REGIONAL TIER GUARDRAIL ANCHOR UNIT, TYPE III GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO STRUCTURE ANCHOR UNITS STATE OF NORTH CAROLINA STATE OF ENGLISH DETAIL DRAWING FOR ENGLISH DETAIL DRAWING FOR BEAM BLOCK BEAM POST "9-,L **JARIABLE** THRIE THRIE OFFSET " pt7 | "8/27 "417 "8°87 STD. 6'-3" SPACING
TRANSTION THE GUARDRAIL VERTICALLY FROM
1'-11" DOWN TO 1'-9" IN ONE 25' SECTION OF 34" DIA **T**0 POST AND OFFSET BLOCK (SECTION WILL REQUIRE BOLT HOLE DRILLING IN IE BEAM OFFSET BLOCK IE POST. 3,-2,, III FOR ATTACHMENT REGIONAL TIER SECTION OF BEAM POST WTR SECTION ELEVATION VIEW 12" GUARDRAIL SHOULDER BREAK

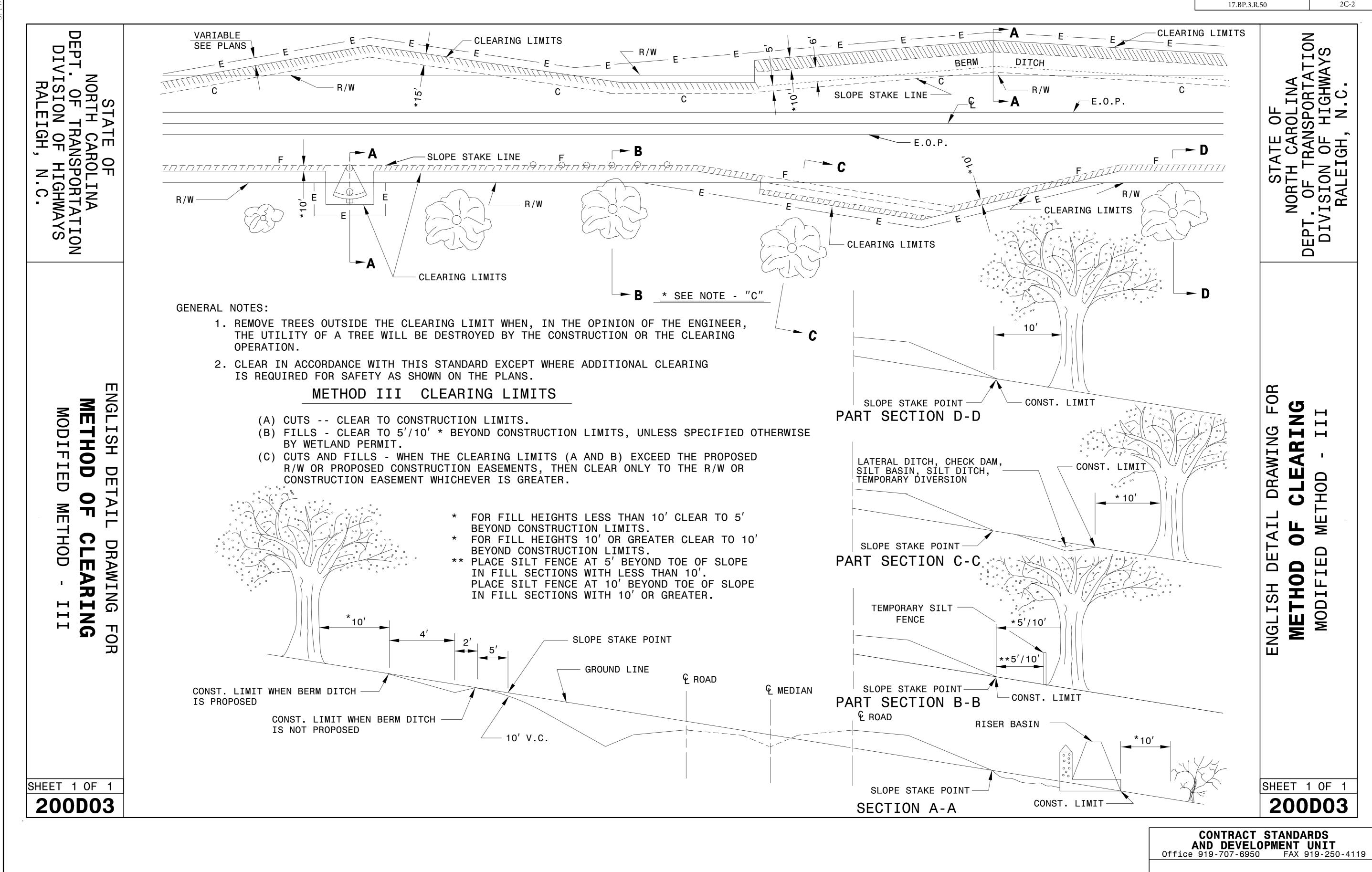
4 " LIP CURB
STRUCTURE PLANS ,,0-,9 THE MID F THE WTR S SPECIAL E THE THRIE AND LINE 5, - 6^{3/9},, SECTION OF WTR BEAM POST 8 3,-2,, TYPE SUB ω v WTR RIDGE OPT 4 IL ANCHOR RAIL ON BE S N 1 ,,0-,9 SLOT (TYP. TO RAIL SE 2'-6" 7,-6,, SECTION OF THRIE BEAM POST 7 1" DIA. HOLES (TYP. FOR ANCHOR BOLTS 78"x 118" FOR UNION 315/ 213/6/ 313/6/ ,,0-,9 10" 10" 50,, THRIE \\\ \L \- \ \ \ "8-'r THRIE-BEAM SECTION SECTION OF POSTS 1 "p\E "8\I "p\E ۷, - 0 STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C. ENGLISH DETAIL DRAWING FOR STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C. ENGLISH DETAIL DRAWING FOR 862d03 STRUCTURE ANCHOR UNITS STRUCTURE ANCHOR UNITS GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO RAIL ON BRIDGE - SUB REGIONAL TIER GUARDRAIL ANCHOR UNIT, TYPE III

CONTRACT STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

ORIGINAL BY: J HOWERTON DATE: 06-22-12

MODIFIED BY: DATE: DATE: FILE SPEC.:



SEE TITLE BLOCK

PROJECT REFERENCE NO.

ORIGINAL BY:	T.S.S.	DATE: _	FEB.2000
MODIFIED BY:		DATE: _	AUG.2016
CHECKED BY:		DATE:	
TIF SPFC : kkem	pf/english/02	00d301 dgn	

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. SHEET NO. 17BP.3.R.50 3B–1

SUMMARY OF EARTHWORK

STATION	STATION	UNCL. EXCAV.	EMBANK. +%	BORROW	WASTE
L STA 16+50.00	-L- STA 17 + 86.88(BRIDGE)	58	104	46	
–L– STA 18+59.13(BRIDGE)	_L_ STA 20+00.00	394	230		164
TOTALS:		452	334	46	164
WASTE IN LIE	U OF BORROW			-46	-46
PROJEC	T TOTALS:	452	334	0	118
GRANE	TOTALS:	452	334	0	118
SAY:		460			118

Note: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, Clearing and Grubbing, and Removal of Existing Pavement will be paid for at the contract lump sum price for "Grading."

PAVEMENT REMOVAL SUMMARY

SURVEY LINE	STATION	STATION	LOCATION LT/RT/CL	YD ²
-	16 + 50.00	17 + 92.82	CL	317
	18 + 48.26	20+00.00	CL	331
			TOTAL:	648
			SAY:	650

SHOULDER BERM GUTTER SUMMARY

SURVEY LINE	STATION	STATION	LENGTH (FT)
-L-	17 + 61.88 RT	17 + 76.00 RT	14.12
	17 + 61.88 LT	17 + 76.00 LT	14.12
		TOTAL:	28.24
		SAY:	30

ROW AREA DATA SUMMARY

	1000 111			5 6 7 7 1 7 1		
PARCEL NO.	PROPERTY OWNERS NAMES	PROP. R⁄W	PERM. UTILTIY EASE.	PERM. DRAIN. EASE.	PERM. DRAINAGE UTILITY EASE.	CONST. EASE.
1	TONY INGRAM MATTHIS AND WIFE					1416.00 S.F.
2	J. KEITH JONES AND WIFE LEANNE B.			500.00 S.F.		
3	R. FLETCHER PEARSON			680.00 S.F.		2040.00 S.F.
4	RONALD CLAYTON MATTHIS & TONY I.			750.00 S.F.		3109.00 S.F.

Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

STATION	N (LT,RT, OR CL)	STRUCTURE NO.	ATION	LEVATION	LEVATION	RITICAL		CAAP			ВІТІ	UMINOUS (UNLESS	COATED	C.S. PIPE OTHERW	: TYPE B (SE)		A	LUMINIZE	SS III R.C OR ED C.S. P OR PIPE, TYPE	PIPE, TYPE				STD. 8 (UN NO	338.01, 838.11 DR 338.80	*	QUANTITY SHALL BE COL. 'A' + (1.3 X COL.'B')	D. 840.02	FRAME, GRATES AND HOOD STANDARD 840.03	STD. 840.15	. 840.16	0.17 OR 840.26 0.18 OR 840.27	.19 OR	GRATE STD. 840.22	GRATE STD. 840.2	H TWO GRATES STD. 840.24 10.32	B' STD. 840.35	D TWO GRATES STD. 840.29			STD. 8	C.B. N.D.I. D.I. G.D.I. G.D.I. (CATCH NARRO DROP GRATE	EVIATIONS H BASIN OW DROP INLET INLET ED DROP INLET ED DROP INLET SOW SLOT)
SIZE	LOCATIO		TOP ELEV	NVERT E	INVERT EL	STOPE O	15" 18" 2	24" 30"	36" 42"	48" 12"	15" 18"	24"	30″	36"	42"	48"	12"	5" 18"	24" 30"	36" 42"	48"	PIPE	PIPE PIPE	CU.	YDS.	RU 5.0′)	В	OR ST		OR S	RATE ST	STD. 84	STD. 840.	VITH GR	AME WITI	AME WITI	., TYPE 'I	AME AN		BOWS N	K PIPE PI	J.B.		TION BOX
THICKNESS OR GAUGE	_	FROM		_				.064	.109	.064	.064	.064	620.	.079	.109	.109						<u>z</u>	SIDE DRAIN		C.S.P.	EACH (0' TH		B. STD. 840.01	TYPE OF GRATE	.l. STD. 840.14	I. FRAME &	G.D.I. TYPE "A" G.D.I. TYPE "B"		G.D.I. FRAME W	G.D.I. (N.S.) FRA	G.D.I. (N.S.) FR.	TB GRATED D.I	.B.D.I. (N.S.) FR		CORR. STEEL EL	CONC. & BRIC	PIPE REMOVAL T.B.J.B.	. TRAFFI	FIC BEARING DROP INLET
						\perp																15″	18″	· •		PER 5.0′		Ü.	E F G	ا ا							· -	-			+		RE <i>^</i>	MARKS
L 17 + 65.00)401)401 0402	72.72	69.97	68.41													24								1											1	1						
L 17 + 65.00		402	72.72	07.77	00.41													24								1											1	1						
		402 OUT		68.41	67.53													16																										
						+																																						
																																								_				
TOTAL																		40								2											2	2						

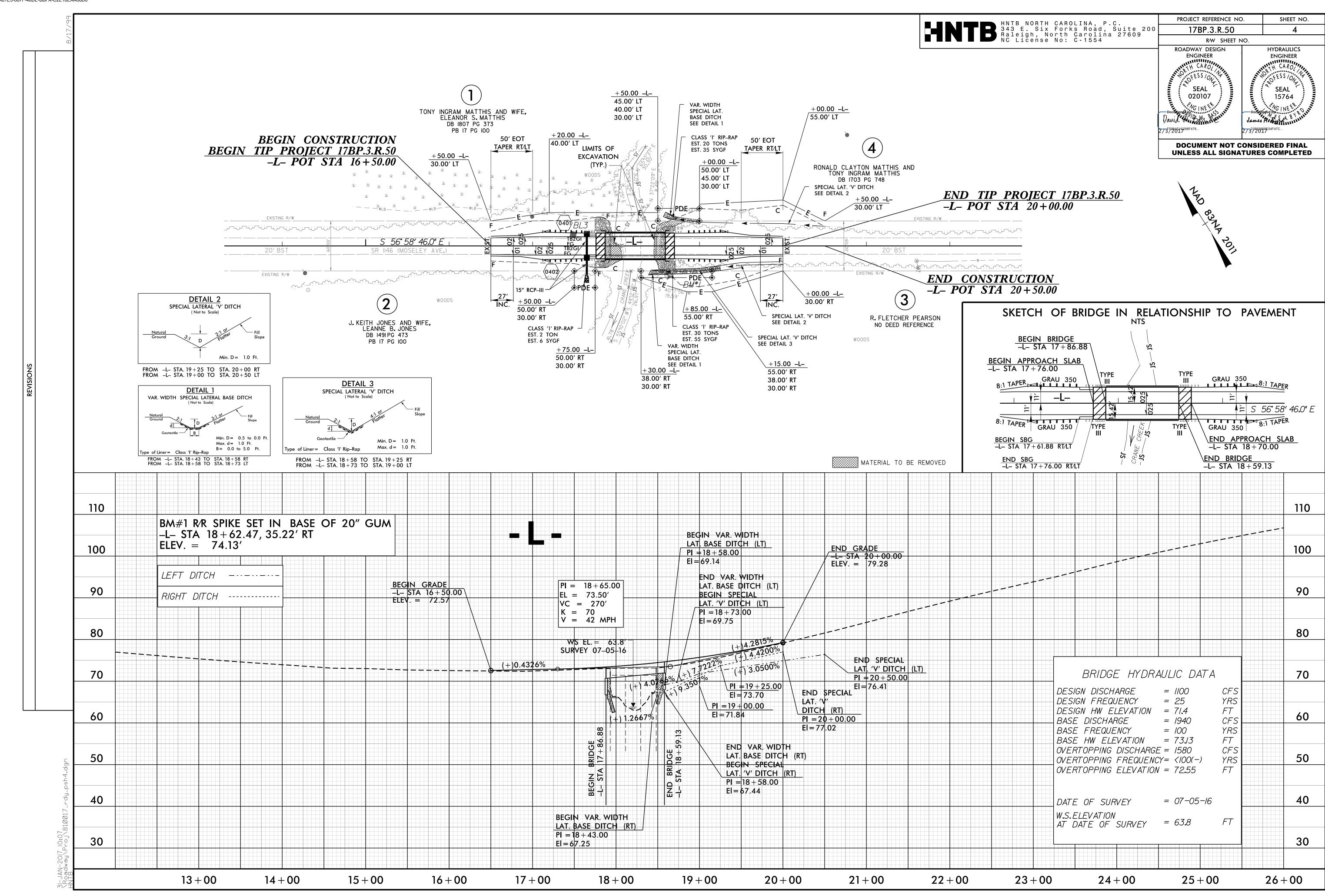
"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL. TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL. W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.

G = GATING IMPACT ATTENUATOR TYPE 350 NG = NON-GATING IMPACT ATTENUATOR TYPE 350

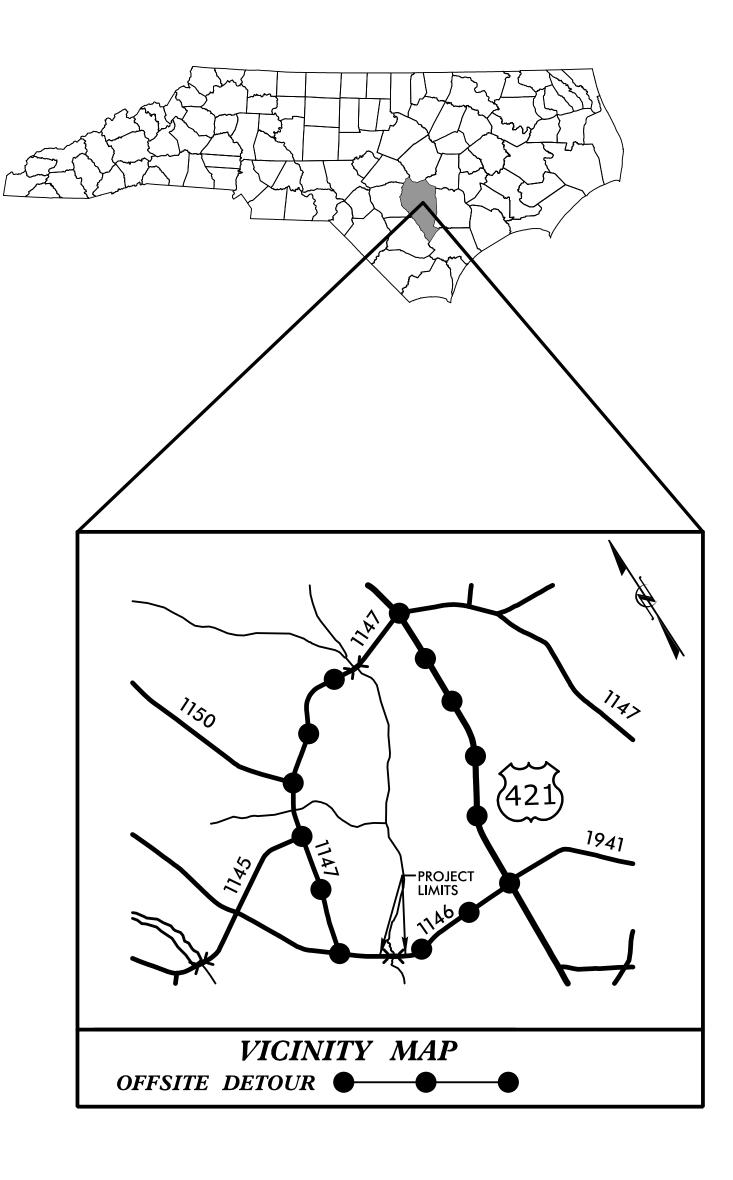
GUARDRAIL SUMMARY

SURVEY LINE BEG. STA.	END STA.	LOCATION		LENGTH		WARRA	WARRANT POINT		TOTAL	FLARE L	FLARE LENGTH		W		ANCHORS						IMPACT ATTENUATO		SINGLE FACED	REMOVE	REMOVE AND		
LINE BEG. STA.	END STA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM SHOUL. E.O.L. WIDTH		APPROACH END	TRAILING END	APPROACH END	TRAILING END	XI MOD	TYPE III	GRAU 350	M-350	XIII	CAT-1	VI MOD	BIC	AT_1	TYPE 350 EA G NG	-1 I	REMOVE EXISTING GUARDRAIL	STOCKPILE EXISTING GUARDRAIL	REMARKS
-L- STA 17+11.88	STA 17 + 86.88(BRIDGE)	RT	75′			STA 17 + 86.88(BRIDGE)		4.42′	7.42′	50′		1′			1	1											
STA 17+11.88	STA 17 + 86.88(BRIDGE)	LT	75′				STA 17 + 86.88(BRIDGE)	4.42′	7.42′		50′		1′		1	1											
STA 18 + 59.13(BRIDGE)	STA 19+34.13	RT	75′				STA 18 + 59.13(BRIDGE)	4.42′	7.42′		50′		1′		1	1											
STA 18+59.13(BRIDGE)	STA 19+34.13	LT	75′			STA 18 + 59.13(BRIDGE)		4.42′	7.42′	50′		1′			1	1											
		SUBTOTAL:	300′												4	4											
	ANCI	HOR DEDUCTIONS	:																								
	(GRAU 350: 4@50'	–200 ′																								
		TYPE III:4@18.75'	–75 ′																								
		TOTAL:	25′																								
		SAY:	37.5′												4	4											
	5	ADDITIONAL POS	П																								



TRANSPORTATION MANAGEMENT PLAN

SAMPSON COUNTY



LOCATION: REPLACE BRIDGE NO. 17 OVER CRANE CREEK ON SR 1146 (MOSELEY AVENUE)

WORK ZONE SAFETY & MOBILITY
"from the MOUNTAINS to the COAST"

N.C.D.O.T. WORK ZONE TRAFFIC CONTROL

1561 MAIL SERVICE CENTER (MSC) RALEIGH, NC 27699-1561

750 N. GREENFIELD PARKWAY, GARNER, NC 27529 (DELIVERY)

PHONE: (919) 773-2800 FAX: (919) 771-2745

JESSI LEONARD, PE DIVISION TRAFFIC ENGINEER



INDEX OF SHEETS

SHEET NO.

<u>TITLE</u>

TMP-1

TITLE SHEET, VICINITY, INDEX OF SHEETS AND LIST OF APPLICABLE ROADWAY STANDARD

DRAWINGS

TMP-2

TEMPORARY TRAFFIC CONTROL PHASING,

GENERAL NOTES AND DETOUR

ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C. DATED JAN 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO. TITLE

1101.03	TEMPORARY ROAD CLOSURES
1101.11	TRAFFIC CONTROL DESIGN TABLES
1110.01	STATIONARY WORK ZONE SIGNS
1145.01	BARRICADES
1205.01	PAVEMENT MARKINGS - LINE TYPES & OFFSETS
1205.02	PAVEMENT MARKINGS - 2 LANE & MULTILANE ROADWAYS
1205.12	PAVEMENT MARKINGS - BRIDGES
1250.01	RAISED PAVEMENT MARKERS - INSTALLATION SPACING
1251.01	RAISED PAVEMENT MARKERS - PERMANENT AND TEMPORARY
1261.01	GUARDRAIL AND BARRIER DELINEATOR SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATOR TYPE
1262.01	GUARDRAIL END DELINEATION

R. B. EARLY, PE

R. B. EARLY, PE

TRAFFIC CONTROL PROJECT ENGINEER

J. A. PHILLIPS

TRAFFIC CONTROL DESIGN ENGINEER

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



HNTB NORTH CAROLINA, P.C. 343 E. Six Forks Road, Ste 200 Raleigh, North Carolina 27609 NC License No: C-1554

APPROVED:

Rhonda B. Early

F34CAF5AC6BF48A...

DATE:

1/31/2017

DATE:

SEAL

023521

SEAL

TIP PROJEC

SHEET NO.

TMP-1

PROJ. REFERENCE NO.	SHEET NO.
17BP.3.R.50	TMP-2

GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN THE DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATIONS MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL THE TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

LANE AND SHOULDER CLOSURE REQUIREMENTS

A) REMOVE LANE CLOSURE DEVICES FROM THE LANE WHEN WORK IS NOT BEING PERFORMED BEHIND THE LANE CLOSURE OR WHEN A LANE CLOSURE IS NO LONGER NEEDED OR AS DIRECTED BY THE ENGINEER.

TRAFFIC PATTERN ALTERATIONS

B) NOTIFY THE ENGINEER TWENTY ONE (21) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

SIGNING

C) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.

PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN ON THIS SHEET.

D) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.

COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.

E) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

TRAFFIC CONTROL DEVICES

F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE **ROADWAY**.

PAVEMENT MARKING AND MARKERS

G) INSTALL PAVEMENT MARKINGS ON THE FINAL SURFACE AS FOLLOWS:

ROAD NAME MARKING MARKERS SR 1146 (MOSELEY RD) **PAINT** RAISED

- H) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.
- I) REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS.
- J) PASSING ZONE WILL BE DETERMINED IN THE FIELD AND MUST BE APPROVED BY THE ENGINEER.

PHASING

PHASE I

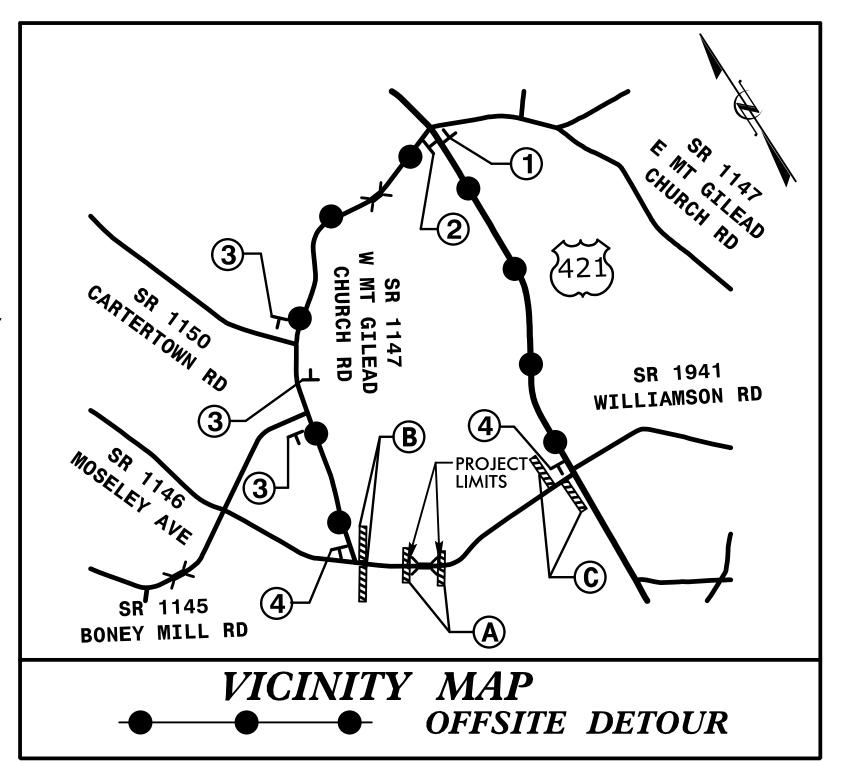
PRIOR TO ANY CONSTRUCTION OPERATIONS, PLACE AND COVER OFF-SITE DETOUR SIGNS AS SHOWN AND IN ACCORDANCE WITH RSD 1101.03 (SHEETS 1 OF 9).

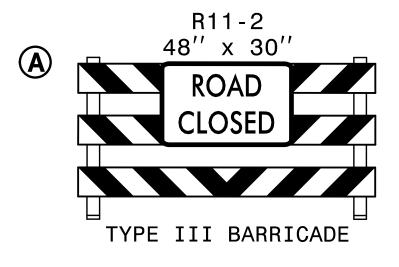
PHASE II

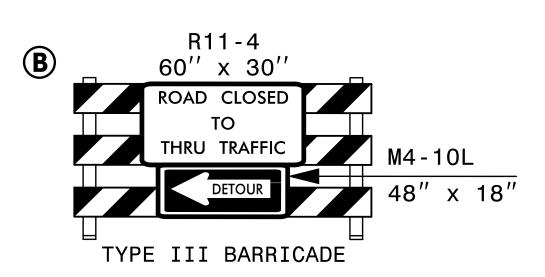
USING OFF-SITE DETOUR, UNCOVER DETOUR SIGNS, CLOSE -L- (SR 1146 / MOSELEY AVE) TO TRAFFIC AND CONSTRUCT BRIDGE, APPROACHES AND ROADWAY UP TO AND INCLUDING THE FINAL LAYER OF SURFACE COURSE.

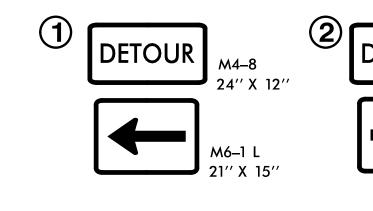
PHASE III

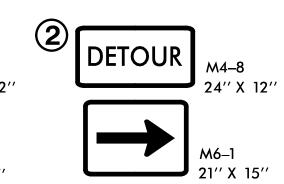
UPON COMPLETION OF BRIDGE, APPROACHES AND ROADWAY, PLACE FINAL PAVEMENT MARKINGS AND MARKERS IN ACCORDANCE WITH RSD 1205.01, 1205.02, 1205.12, 1250.01 AND 1251.01. REMOVE BARRICADES AND DETOUR SIGNS AND OPEN -L- (SR 1146 / MOSELEY AVE) TO TRAFFIC.

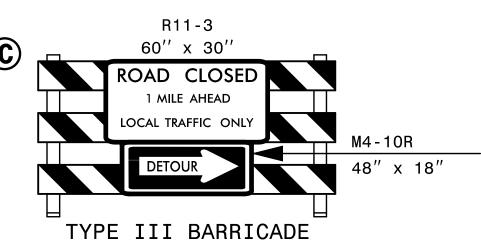


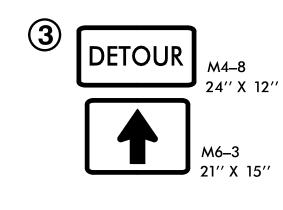




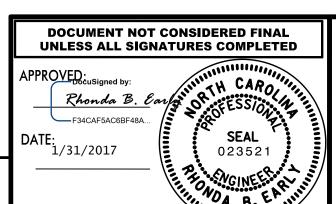


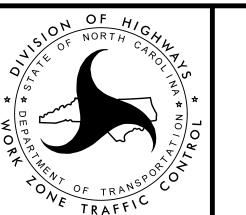












PHASING, PROJECT NOTES, AND DETOUR

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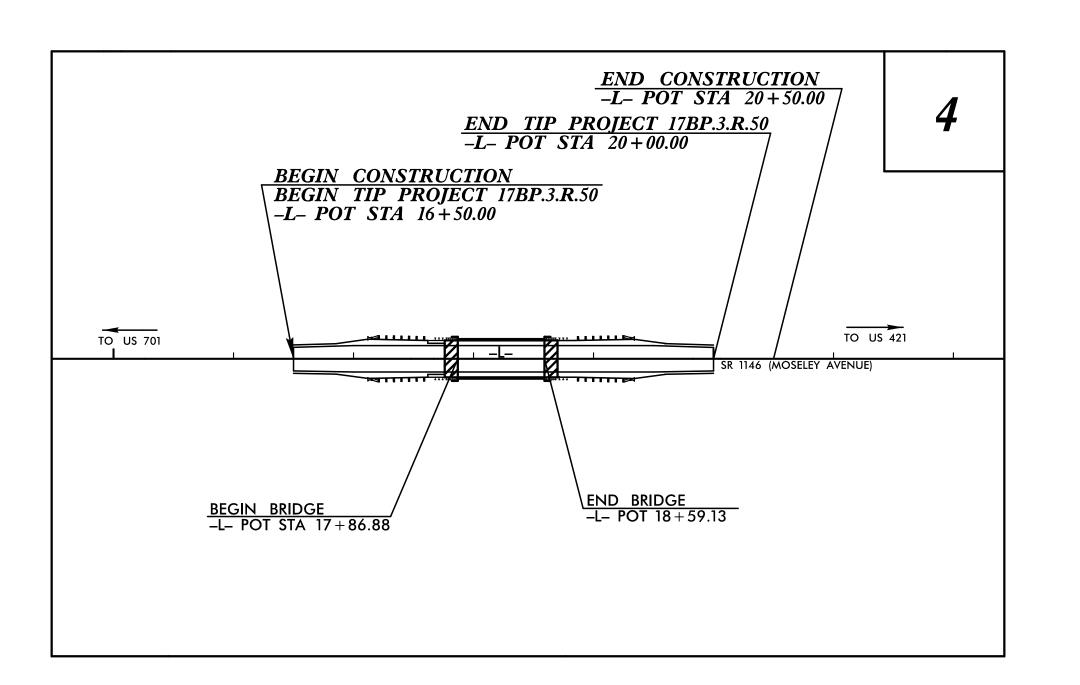
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

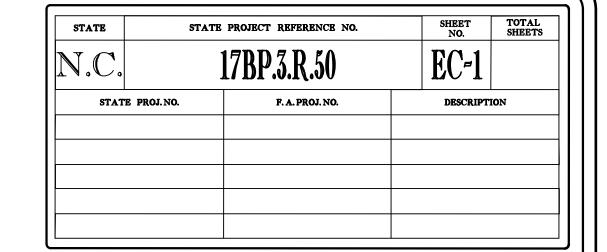
PLAN FOR PROPOSED HIGHWAY EROSION CONTROL

SAMPSON COUNTY

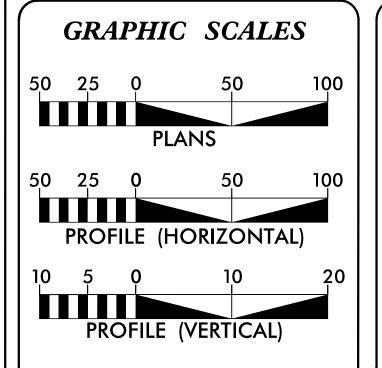
LOCATION: REPLACE BRIDGE NO. 17 OVER CRANE CREEK ON SR 1146 (MOSELEY AVENUE)

TYPE OF WORK: GRADING, DRAINAGE, PAVING AND STRUCTURE





EROSIO	N AND SEDIMENT CONTROL MEASURES
<u>Séd.</u> #	Description Symbol
1630.03	Temporary Silt Ditch
1630.05	Temporary Diversion
1605.01	Temporary Silt Fence
1606.01	Special Sediment Control Fence
1622.01	Temporary Berms and Slope Drains
	Silt Basin Type B
1633.01	Temporary Rock Silt Check Type-A
	Temporary Rock Silt Check Type A with Matting and Polyacrylamide (PAM)
	Temporary Rock Silt Check Type-B
	Wattle / Coir Fiber Wattle
	Wattle / Coir Fiber Wattle with Polyacrylamide (PAM)
1634.01	Temporary Rock Sediment Dam Type-A
1634.02	Temporary Rock Sediment Dam Type-B
1635.01	Rock Pipe Inlet Sediment Trap Type-A
1635.02	Rock Pipe Inlet Sediment Trap Type-B
1630.04	Stilling Basin
1630.06	Special Stilling Basin
	Rock Inlet Sediment Trap:
1632.01	Туре А
1632.02	Туре В
1632.03	Туре С
	Skimmer Basin
	Tiered Skimmer Basin
	Infiltration Basin



ROADSIDE ENVIRONMENTAL UNIT **DIVISION OF HIGHWAYS** STATE OF NORTH CAROLINA

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 3, 2011 ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

Prepared in the Office of:

HNTB NORTH CAROLINA, P.C.
343 E. Six Forks Road, Suite 200
Raleigh, North Carolina 27609
NC License No: C-1554

2012 STANDARD SPECIFICATIONS

NATALIE CHAN, P.E. **EROSION CONTROL** LEVEL III CERTIFICATION #3444 Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2012 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of

04.01	Railroad Erosion Control Detail
05.01	Temporary Silt Fence
06.01	Special Sediment Control Fence
07.01	Gravel Construction Entrance
22.01	Temporary Berms and Slope Drains
30.01	Riser Basin
30.02	Silt Basin Type B

1630.03 Temporary Silt Ditch

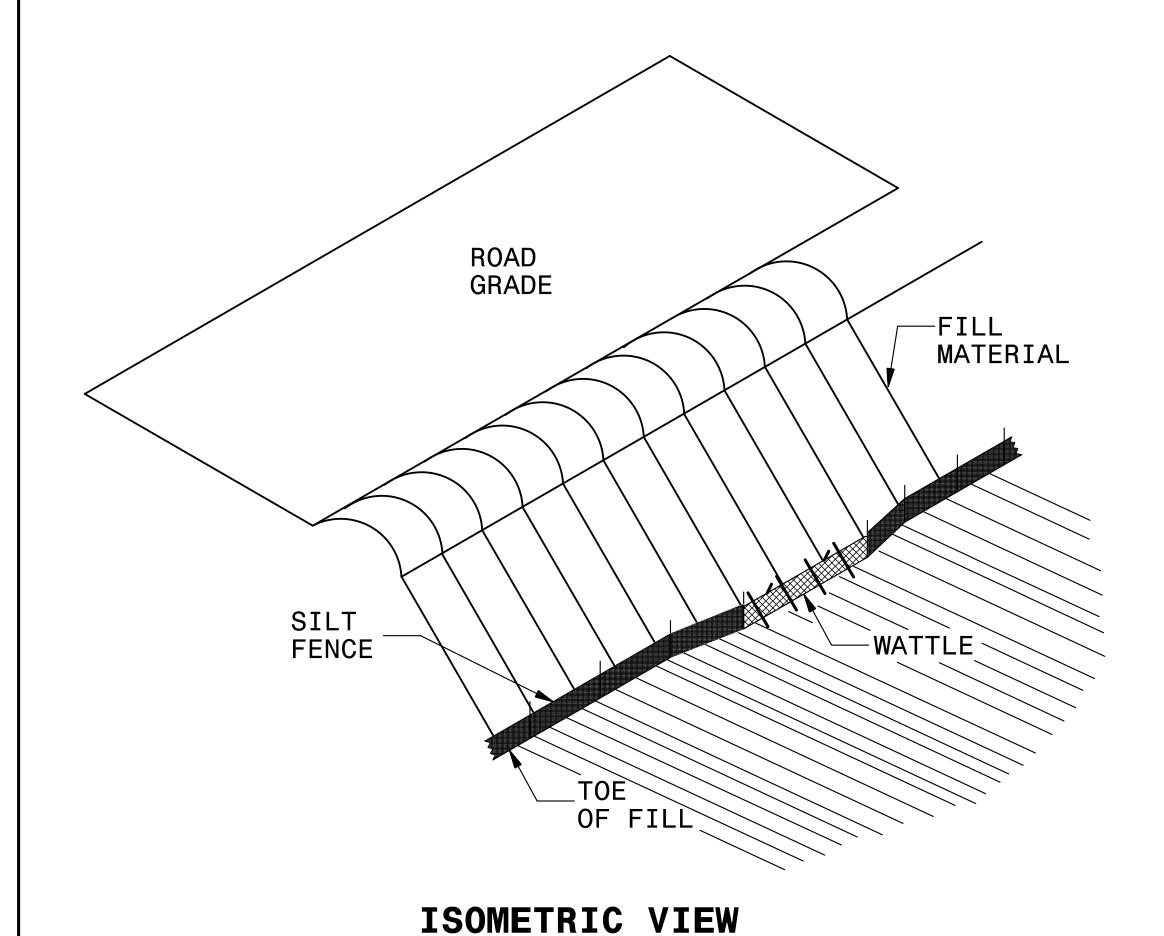
1630.04 Stilling Basin 1630.05 Temporary Diversion 1630.06 Special Stilling Basin 1631.01 Matting Installation

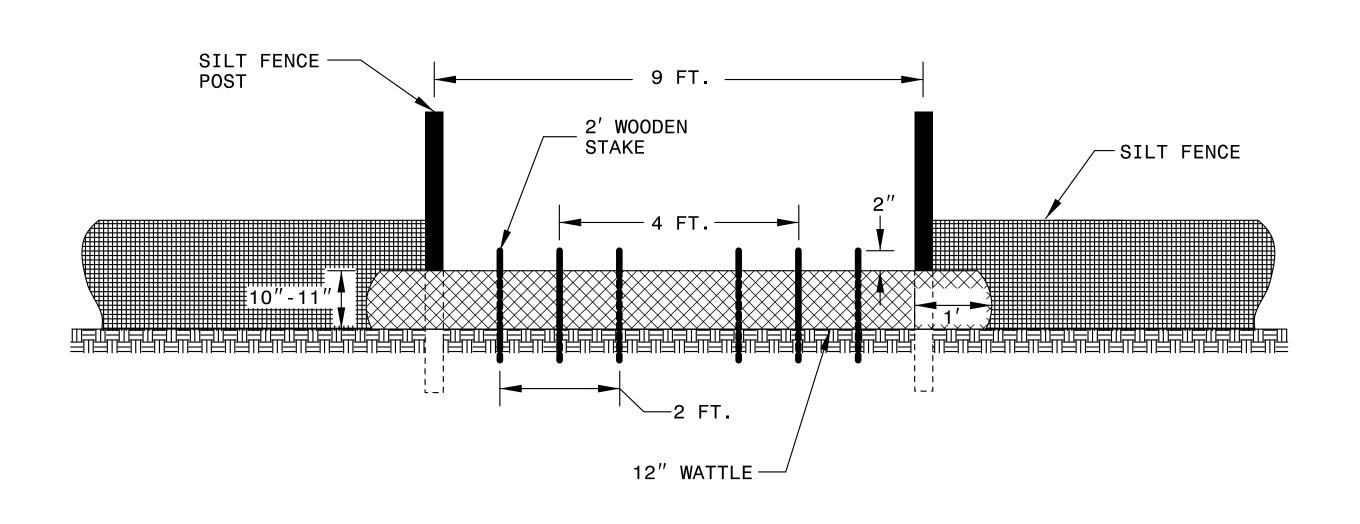
1632.01 Rock Inlet Sediment Trap Type A 1632.02 Rock Inlet Sediment Trap Type B 1632.03 Rock Inlet Sediment Trap Type C 1633.01 Temporary Rock Silt Check Type A 1633.02 Temporary Rock Silt Check Type B 1634.01 Temporary Rock Sediment Dam Type A 1634.02 Temporary Rock Sediment Dam Type B
1635.01 Rock Pipe Inlet Sediment Trap Type A
1635.02 Rock Pipe Inlet Sediment Trap Type B 1640.01 Coir Fiber Baffle 1645.01 Temporary Stream Crossing

 PROJECT REFERENCE NO.
 SHEET NO.

 17BP.3.R.50
 EC-2

SILT FENCE WATTLE BREAK DETAIL





VIEW FROM SLOPE

NOTES:

USE MINIMUM 12 IN. DIAMETER EXCELSIOR WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

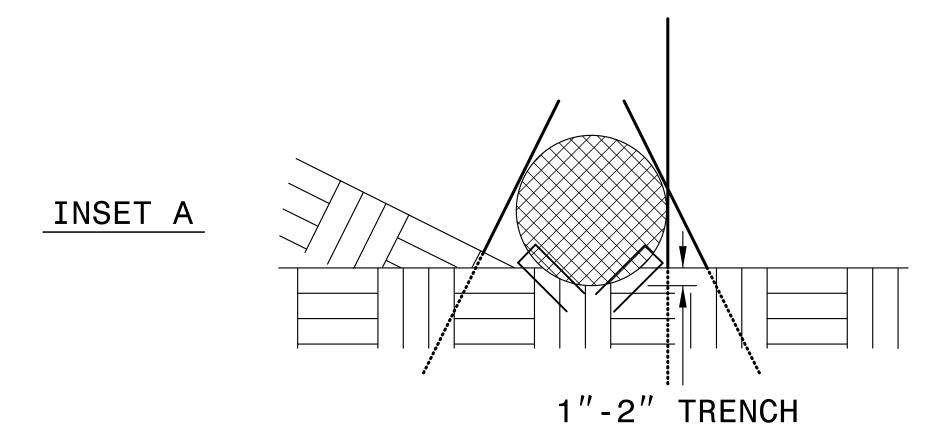
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

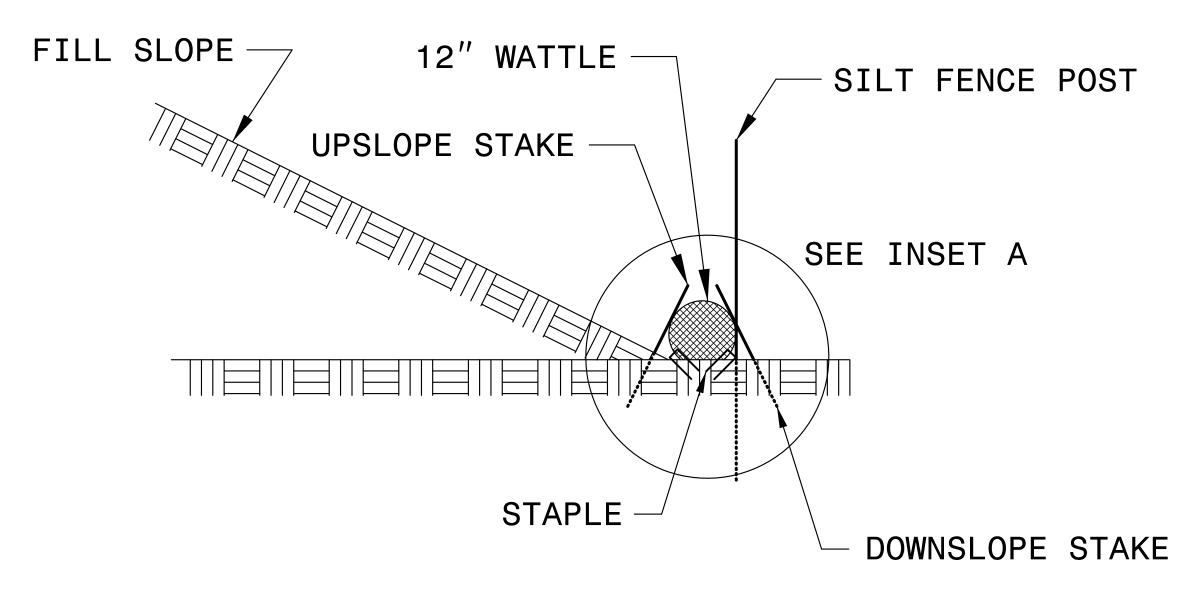
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.





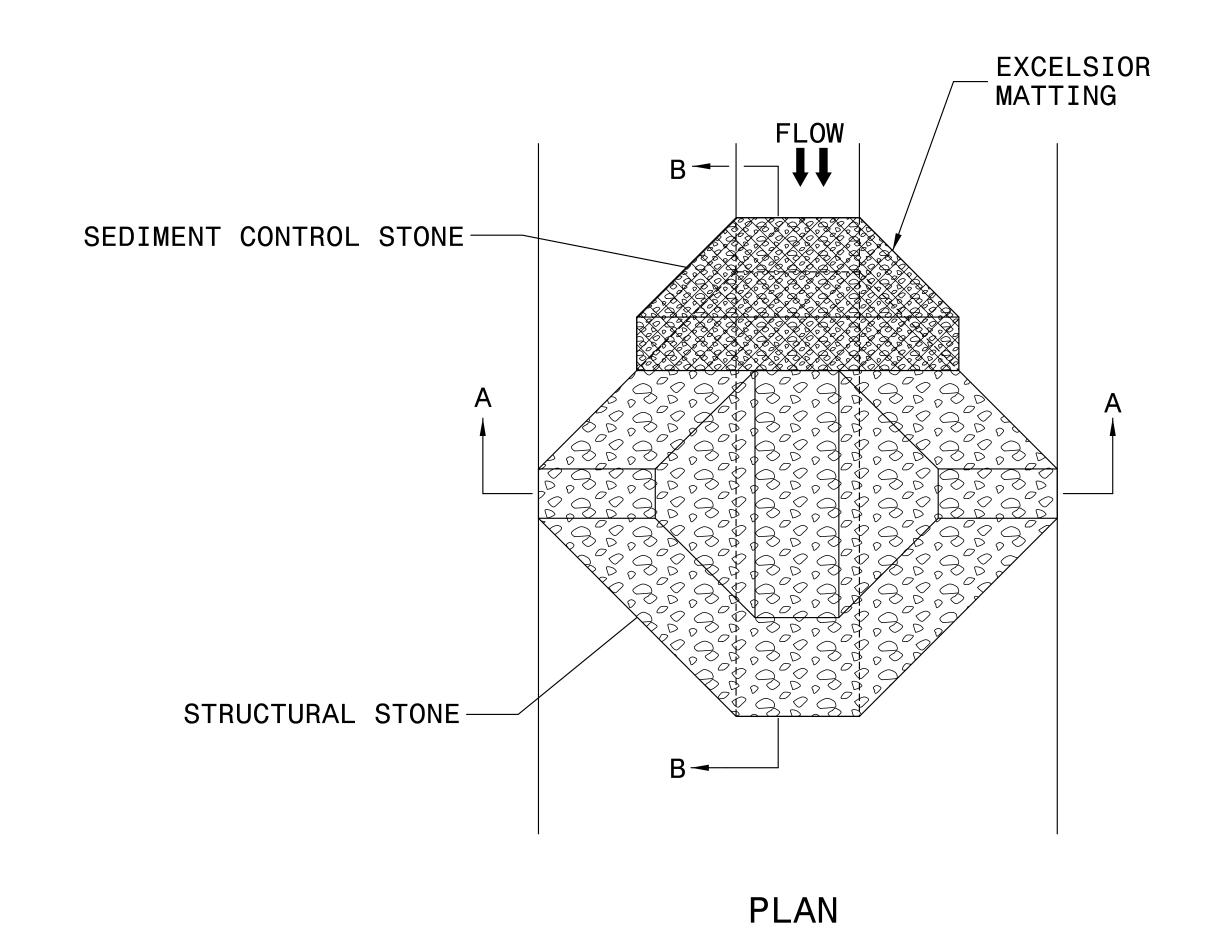
SIDE VIEW

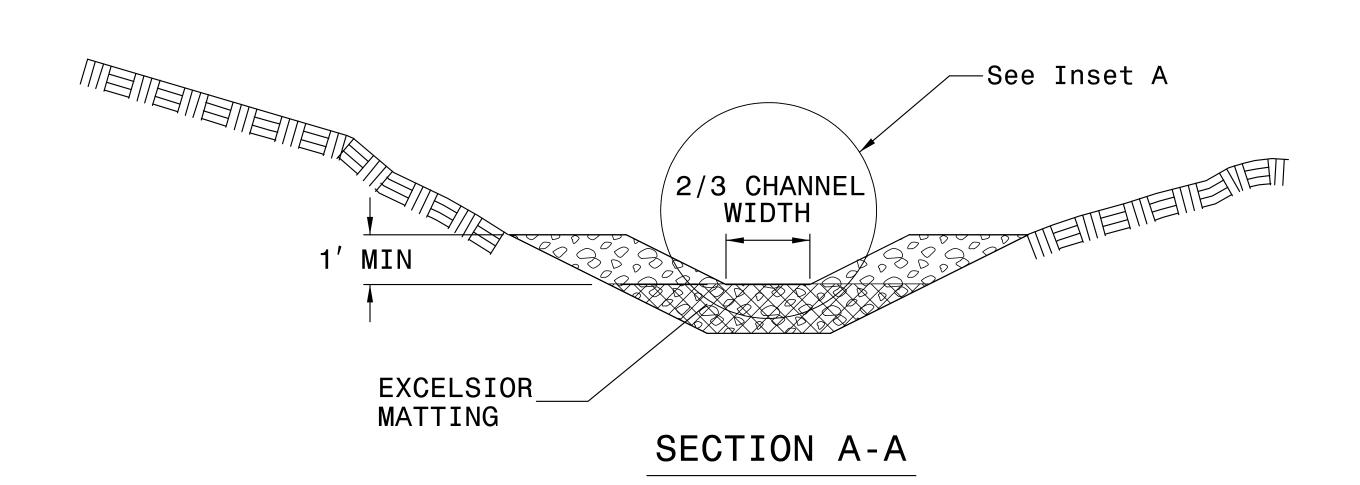
NOT TO SCALE

 PROJECT REFERENCE NO.
 SHEET NO.

 17BP.3.R.50
 EC-2A

TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)





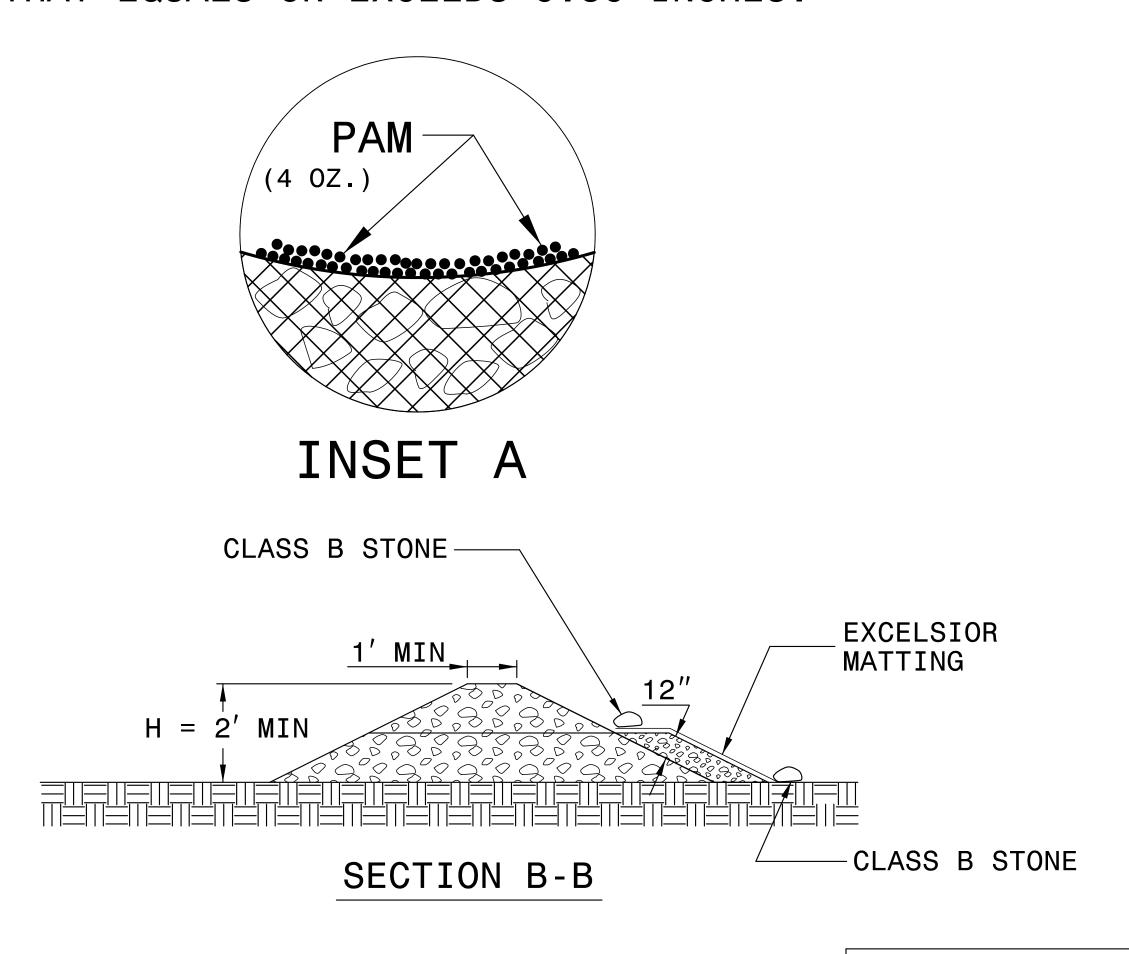
NOTES:

INSTALL TEMPORARY ROCK SILT CHECK TYPE A IN ACCORDANCE WITH ROADWAY STANDARD DRAWING NO. 1633.01.

USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 4 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.



NOT TO SCALE

DJECT REFERENCE NO.	SHEET NO.
17 DD 3 D 50	<i>E</i> ∩_3
// DF).\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	Γ (, - .)

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION SUMMARY SHEET

MATTING FOR EROSION CONTROL

PERMANENT SOIL REINFORCEMENT MAT

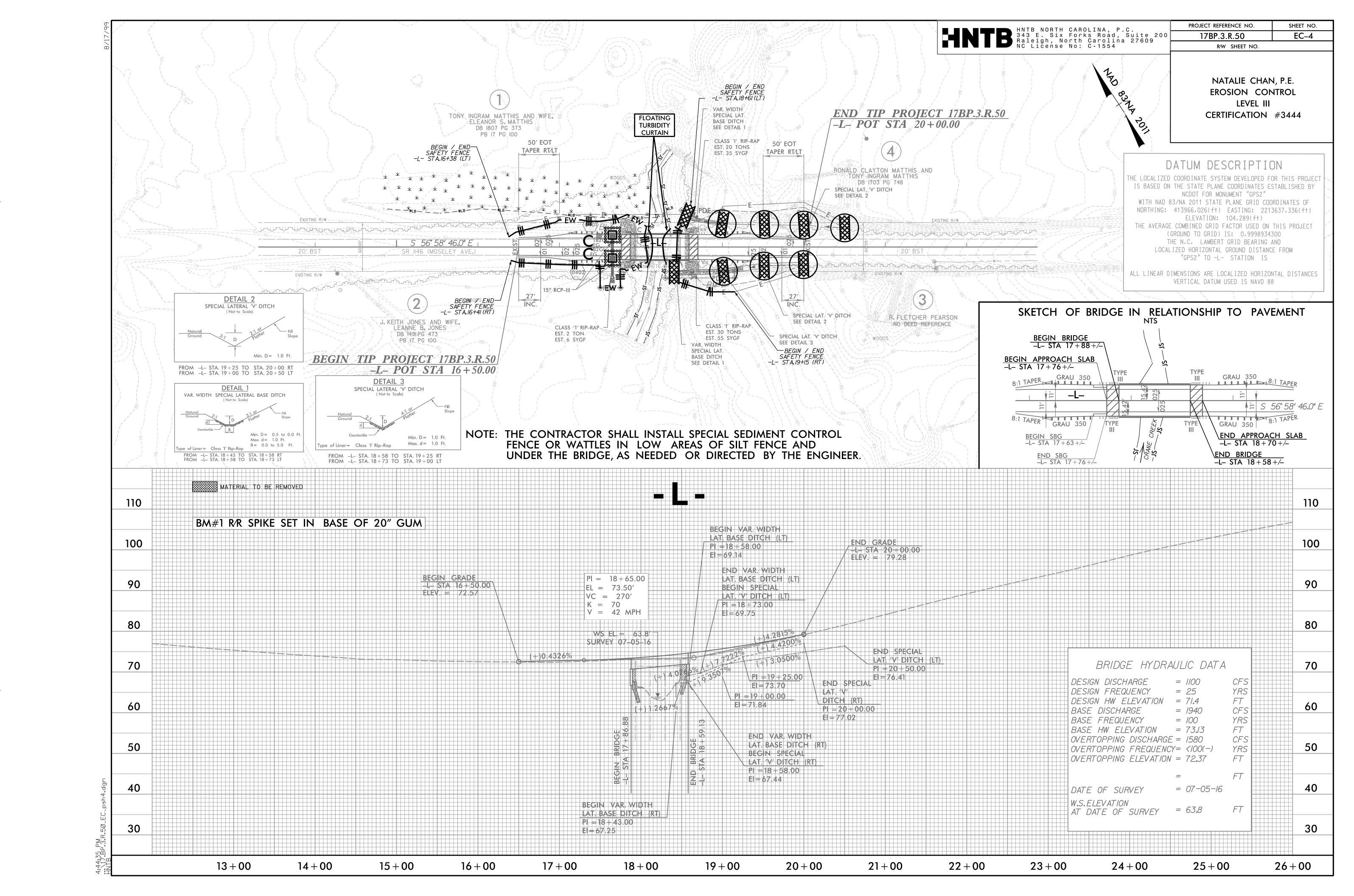
	WILLIAM TON LINOSTON CONTINOL						I ERWIMIERI SOIL REINIORGENIERI MMI						
CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)	CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE	(SY)	
4	L	18+73	2Ø+5Ø	LT	5Ø	4	L	18+58	20+00	RT	25		
			6118	STOTAL	5Ø				61	BTOTAL	25		
MIGCELLANGOUG	MATTING TO BE INSTA	NI 1 60 A6 OIP6	+	+				ADDITIONAL					
MIBUELLKINEDUS	MATITING TO DE TINSTA	ALLER NO VIND			1300			AVVIIIONAV	PONM TO DE				
				TOTAL	1350					TOTAL	25		
				SAY	1350					SAY	25		

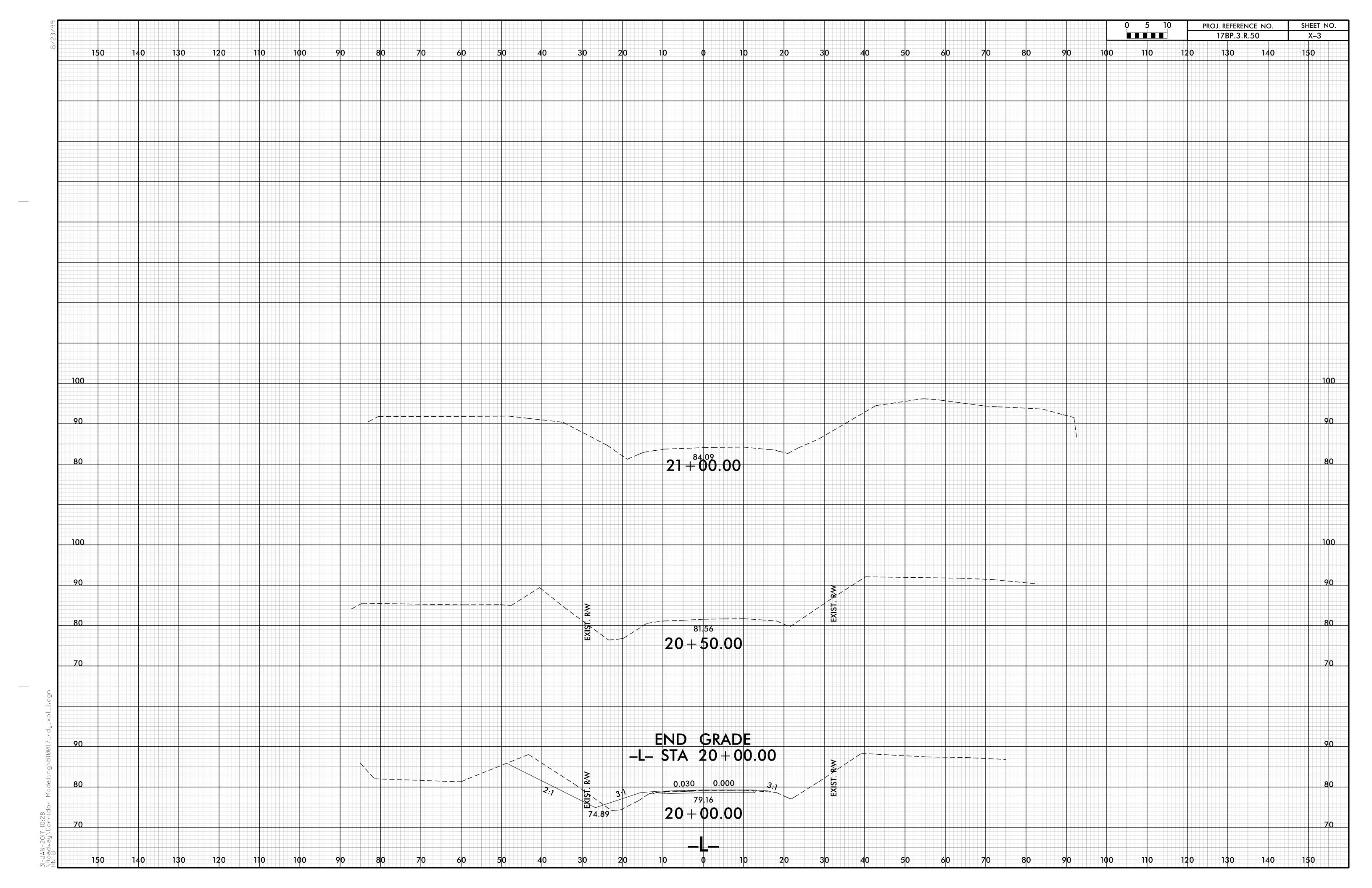
PROJECT REFERENCE NO. SHEET NO. EC-3A

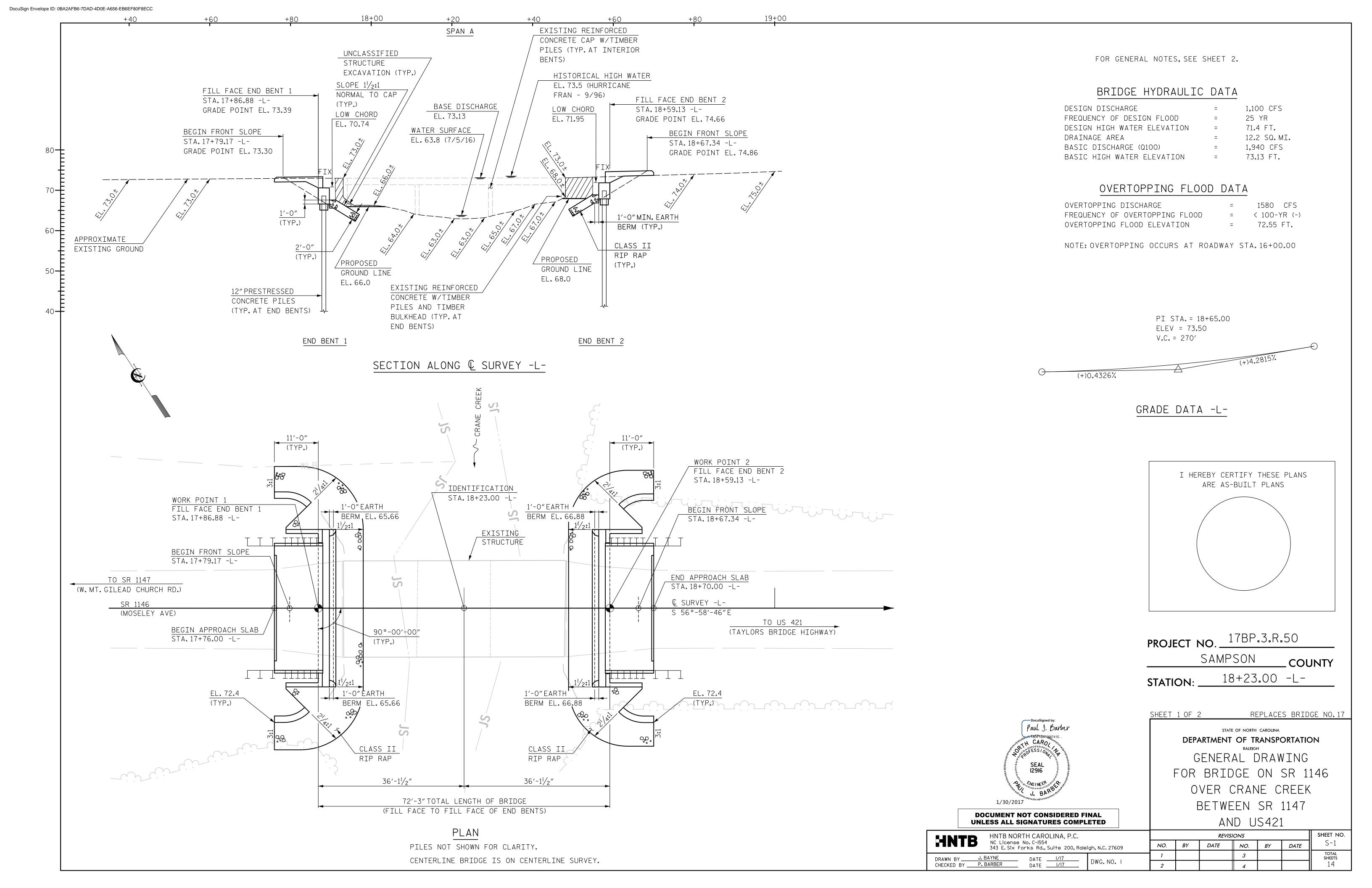
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

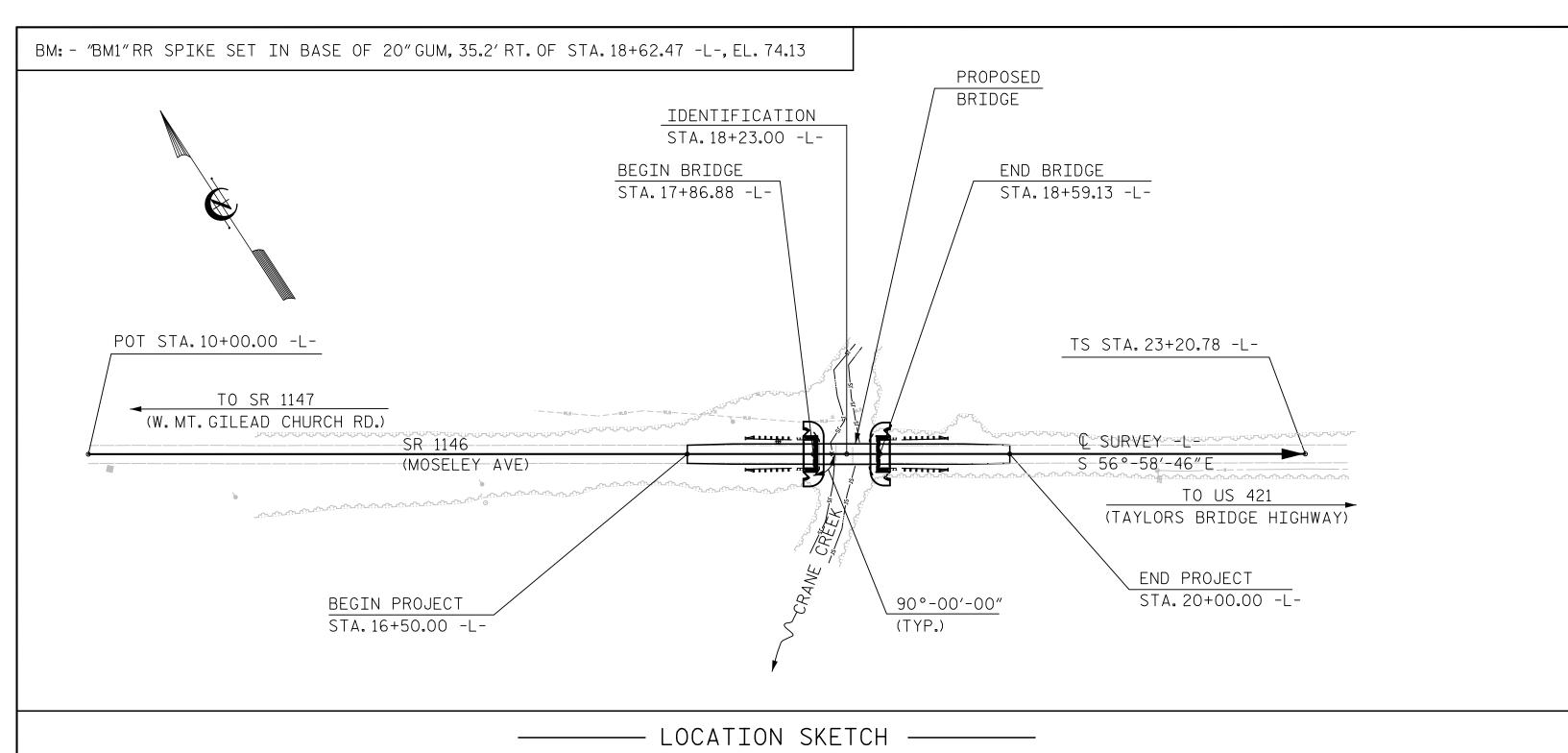
SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	I4 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.









FOR UTILITY INFORMATION. SEE UTILITY PLANS AND SPECIAL PROVISIONS.

FOUNDATION NOTES:

FOR PILES. SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 85 TONS PER PILE.

PILES AT END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 85 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 145 TONS PER PILE.

DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 145 TONS PER PILE.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING. SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

	TOTAL BILL OF MATERIAL													
	REMOVAL OF EXISTING STRUCTURE AT STATION 18+23.00 -L-	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION AT STATION 18+23.00 -L-	CLASS A CONCRETE	BRIDGE APPROACH SLABS AT STATION 18+23.00 -L-	REINFORCING STEEL	12" PRESTRESSED CONCRETE PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0"x2'-0" PRESTRESSED CONCRETE CORED SLABS	ASBESTOS ASSESSMENT
	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO. LIN.FT.	EACH	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO. LIN.FT.	LUMP SUM
SUPERSTRUCTURE	LUMP SUM				LUMP SUM				140.25			LUMP SUM	11 770	
END BENT 1			LUMP SUM	20.6		2,636	7 210	7		150	165			
END BENT 2			LUMP SUM	20.6		2,636	7 245	7		140	155			
TOTAL	LUMP SUM	1	LUMP SUM	41.2	LUMP SUM	5,272	14 455	14	140.25	290	320	LUMP SUM	11 770	LUMP SUM

GENERAL NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THIS BRIDGE SHALL BE CONSTRUCTED USING TOP-DOWN CONSTRUCTION METHODS. THE USE OF A TEMPORARY CAUSEWAY OR WORK BRIDGE IS NOT PERMITTED.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD. THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICAL 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 18+23.00 -L-"

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 19.5 FT. ON EACH SIDE OF CENTERLINE BRIDGE AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING THREE SPAN STRUCTURE WITH SPAN LENGTHS OF 18'-4". 18'-3". AND 18'-6'' WITH 12 LINES OF W 12×16.5 I-BEAMS AT $2'-2\frac{1}{2}''$ CENTERS, WITH A REINFORCED CONCRETE DECK WITH A 25.33' OUT TO OUT DECK WIDTH ON REINFORCED CONCRETE CAPS AND TIMBER PILES (SOME ENCASED IN CONCRETE) SHALL BE REMOVED. IN ADDITION, ANY PILES REMAINING FROM PREVIOUS BRIDGE CONSTRUCTION OR MAINTENANCE OPERATIONS SHALL BE REMOVED AND INCLUDED IN THE LUMP SUM PAY ITEM FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 18+23.00 -L-".

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

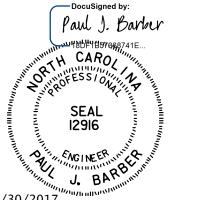
FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, PRESTRESSED CONCRETE END BENT AND BENT CAPS MAY BE SUBSTITUTED IN PLACE OF THE CAST-IN-PLACE CAPS. THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER TO RECEIVE REVISED PLANS AND DETAILS FROM THE STRUCTURES MANAGEMENT UNIT. THE REDESIGN AND ANY MATERIALS NEEDED WILL BE AT NO EXTRA COST TO THE CONTRACTOR.

PROJECT NO. ____17BP.3.R.50 SAMPSON _ COUNTY 18+23.00 -L-

SHEET 2 OF 2

STATION: _



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DEPARTMENT OF TRANSPORTATION GENERAL DRAWING FOR BRIDGE ON SR 1146 OVER CRANE CREEK BETWEEN SR 1147 AND LIS421

STATE OF NORTH CAROLINA

l						<u> </u>	ט ט.	J 121	•	
HNTB NORTH CAROLINA, P.C. NC License No. C-1554					SHEET NO.					
	NC License No. C-1554 343 E. Six Forks Rd., Sui	te 200, Raleigh	, N.C. 27609	NO.	BY	DATE	NO.	BY	DATE	S-2
RAWN BY	YJ.BAYNEDATE	1/17	WO NO O	1			3			TOTAL SHEETS
CHECKED		<u>I/I7</u> D	WG. NO. 2	2			4			14

ASSEMBLED BY: J. BAYNE

DRAWN BY: CVC 6/10

CHECKED BY : DNS 6/10

CHECKED BY: P. BARBER

DATE: 12/16

DATE: 1/17

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT LOCA DISTRIBL FACTORS DISTE FACT(S ΗШ 1.32 1.006 1.75 0.273 1.03 70′ EL 34.5 0.507 70′ 0.80 0.273 1.01 70′ 34.5 HL-93(Inv)N/A 6.9 0.273 70′ 34.5 0.507 1.72 HL-93(0pr) 1.341 1.34 EL 70′ EL 6.9 N/A DESIGN 1.65 LOAD 70′ 70′ EL 0.273 HS-20(Inv) 36.000 47.02 1.75 0.273 EL 34.5 0.507 6.9 70′ EL 34.5 1.306 1.34 0.80 1.31 RATING 36.000 0.273 34.5 0.507 EL HS-20(0pr) 62.64 70′ EL 2.14 70′ 6.9 1.74 1.74 SNSH 13.500 2.917 39.379 0.273 3.75 70′ EL 34.5 0.507 4.87 70′ EL 6.9 0.80 0.273 2.92 70′ EL 34.5 20.000 2.187 43.741 0.273 0.507 3.47 0.273 SNGARBS2 2.81 70′ EL 34.5 70′ EL 6.9 0.80 2.19 70′ 34.5 SNAGRIS2 2.077 45.69 0.273 70′ EL 34.5 0.507 3.23 70′ EL 0.80 0.273 2.08 22.000 2.67 6.9 70′ 34.5 27.250 39.565 0.273 70′ 34.5 0.507 70′ EL 0.273 70′ SNCOTTS3 1.452 1.87 EL 2.43 6.9 1.45 34.5 0.80 34.925 42.554 0.273 70′ EL 34.5 0.507 2.03 70′ EL 0.273 1.22 70′ 34.5 SNAGGRS4 1.218 1.4 1.57 6.9 0.80 35.550 0.273 0.507 2.06 70′ 0.80 0.273 70′ SNS5A 1.191 42.346 1.53 70′ EL 34.5 EL 6.9 1.19 34.5 39.950 0.273 1.88 1.095 43.747 70′ EL 34.5 0.507 70′ EL 6.9 0.80 0.273 1.10 70′ 34.5 SNS6A 1.41 42.000 0.273 70′ EL 34.5 0.507 1.85 70′ EL 0.273 SNS7B 1.043 43.801 1.34 6.9 1.04 70′ 34.5 0.80 LEGAL LOAD 0.273 70′ EL 34.5 0.507 2.23 70′ EL 0.273 70′ 34.5 TNAGRIT3 33.000 1.336 44.087 1.72 6.9 0.80 1.34 1.4 EL RATING 33.075 44.401 0.273 0.507 70′ 2.17 70′ 0.80 0.273 1.342 1.72 EL 34.5 EL 6.9 1.34 70′ 34.5 TNT4A 0.273 34.5 1.98 70′ EL 70′ EL 0.80 0.273 1.10 TNT6A 41.600 1.1 45.746 1.41 0.507 6.9 70′ EL 34.5 42.000 0.273 34.5 0.507 TNT7A 1.106 46.462 1.42 70′ EL 1.94 70′ EL 6.9 0.80 0.273 70′ EL 34.5 1.11 0.273 70′ EL 34.5 0.507 70′ EL 0.273 70′ 34.5 TNT7B 42.000 1.147 48.18 1.47 1.8 6.9 0.80 1.15 EL 1.4 TNAGRIT4 43.000 46.838 0.273 1.4 70′ EL 34.5 0.507 1.74 70′ EL 6.9 0.273 70′ 34.5 0.80 1.09 0.273 0.507 1.74 0.80 0.273 1.026 1.32 70′ 70′ 70′ TNAGT5A 45.000 34.5 1.03 34.5 46.175 1.013 | 45.579 0.507 TNAGT5B 1.01 45.000 0.273 34.5 EL

LRFR SUMMARY

FOR SPAN 'A'

LOAD FACTORS:

	DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
		STRENGTH I	1.25	1.50
		SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

(#) CONTROLLING LOAD RATING

 $\langle 1 \rangle$ DESIGN LOAD RATING (HL-93)

 $\langle 2 \rangle$ DESIGN LOAD RATING (HS-20)

 $\langle 3 \rangle$ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.3.R.50

SAMPSON

COUNTY

SHEET NO.

S-3

TOTAL SHEETS

18+23.00 -L-STATION:_

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

LRFR SUMMARY FOR 70' CORED SLAB UNIT 90° SKEW

(NON-INTERSTATE TRAFFIC)

HNTB NORTH CAROLINA, P.C. NC License No. C-1554

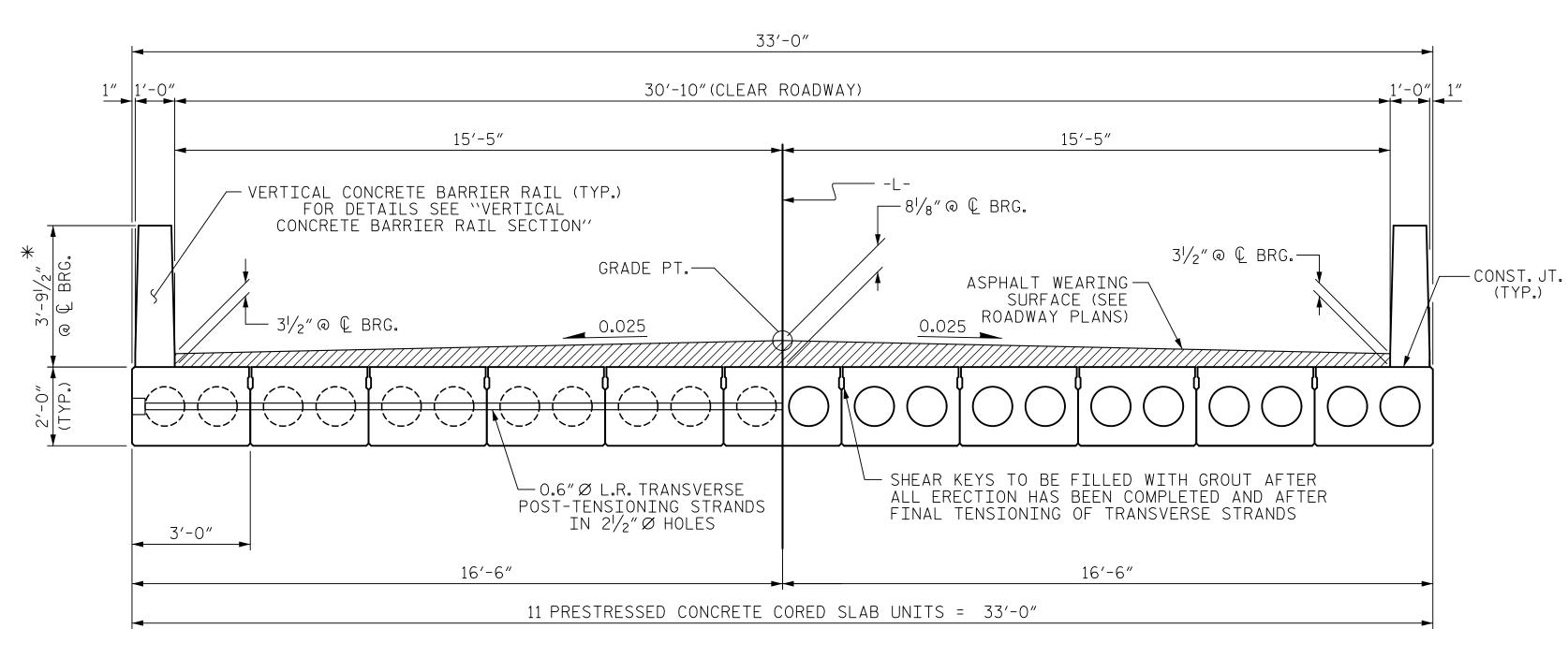
Paul J. Barber SEAL 12916 WGINEE!

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

BY:

REVISIONS DATE: DATE: 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 DRAWN BY J. BAYNE DATE 12/16
CHECKED BY P. BARBER DATE 1/17 DWG. NO. 3

STD. NO. 24LRFR1_90S_70L

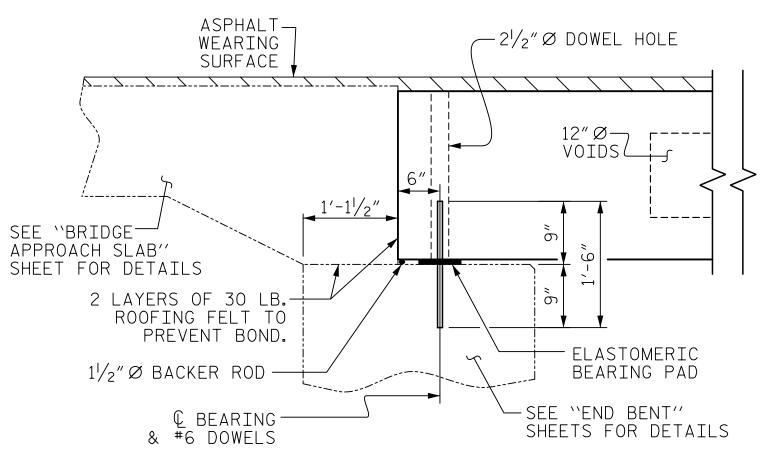


HALF SECTION AT INTERMEDIATE DIAPHRAGMS

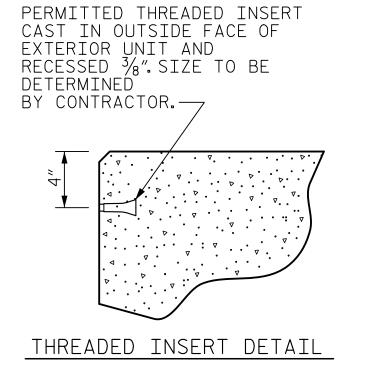
HALF SECTION THROUGH VOIDS

*- THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

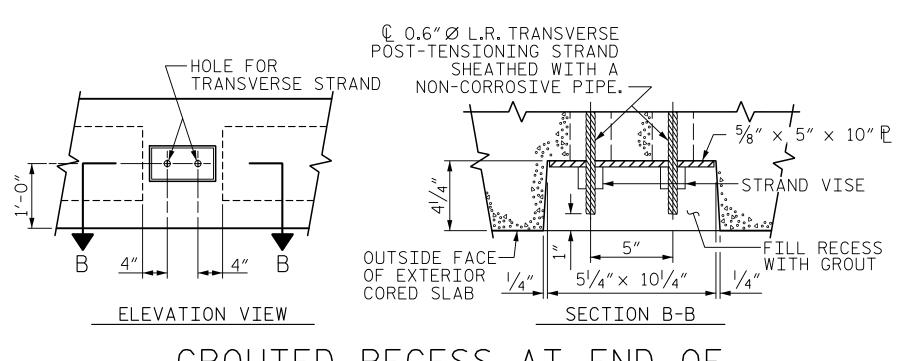
FIXED END



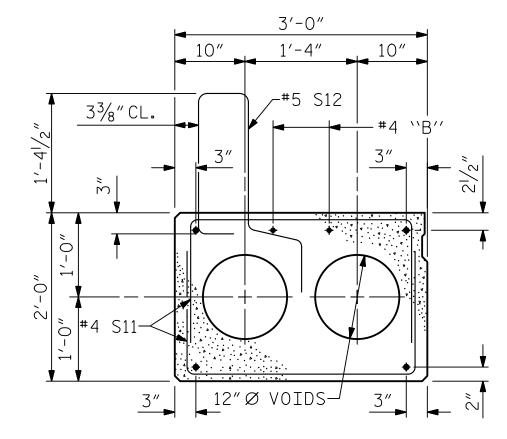
ION AT END BENT



ASSEMBLED BY : J. BAYNE DATE: 12/16 CHECKED BY: P. BARBER DATE: 1/17 DRAWN BY: MAA 6/10 REV. 9/14 MAA/TMG CHECKED BY : MKT 7/10



GROUTED RECESS AT END OF POST-TENSIONED STRAND CORED SLABS



EXTERIOR SLAB SECT

- € 2½″Ø DOWEL HOLES

>─#4 S14

#5 S15

4" | 4"

-1" CL.

SHOWING PLACEMENT OF DOUBLE STIRRUPS

AND LOCATION OF DOWEL HOLES.

(STRAND LAYOUT NOT SHOWN.)

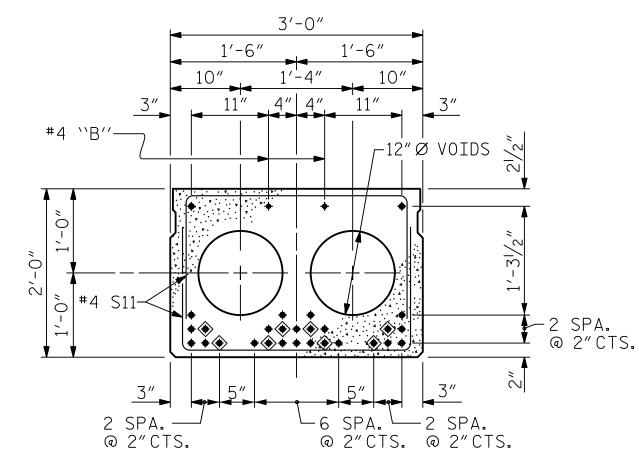
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB

UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

_EVATION

#5 S10—

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

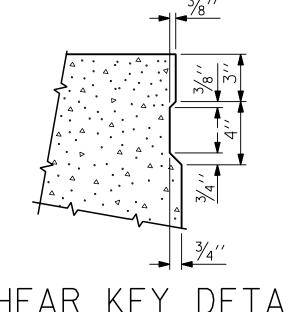


INTERIOR SLAB SECTION (70'UNIT) (28 STRANDS REQUIRED)

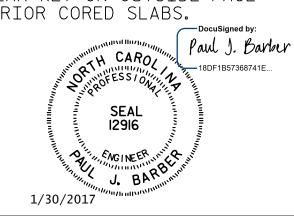
LOW RELAXATION STRAND LAYOUT

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 12'-0"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND



NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

HNTB NORTH CAROLINA, P.C. HNTB NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 DATE <u>| 12/16</u> DATE <u>| 1/17</u>

CHECKED BY _____P.BARBER

PROJECT NO. 17BP.3.R.50

SAMPSON COUNTY

18+23.00 -L-STATION:

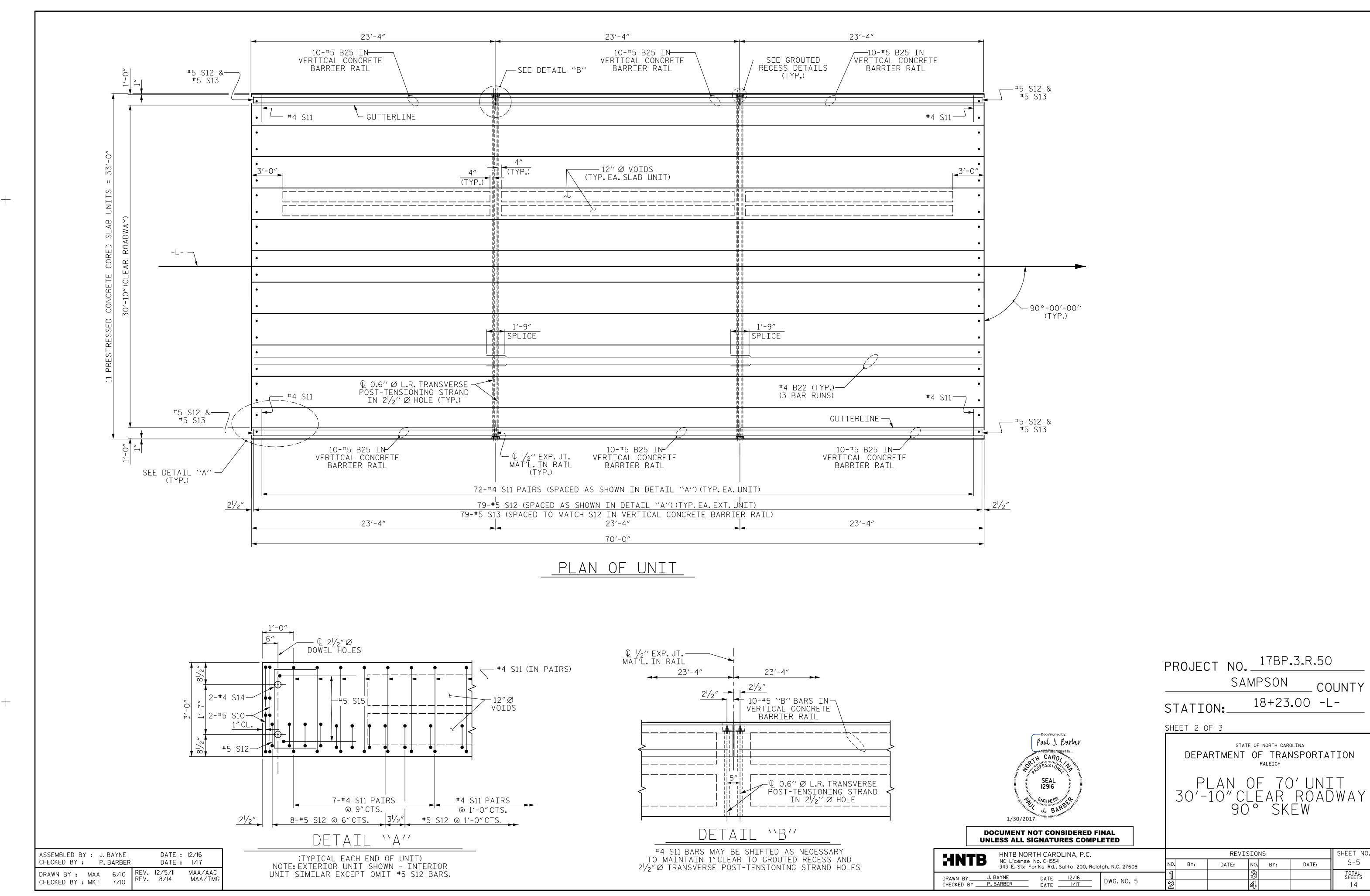
SHEET 1 OF 3

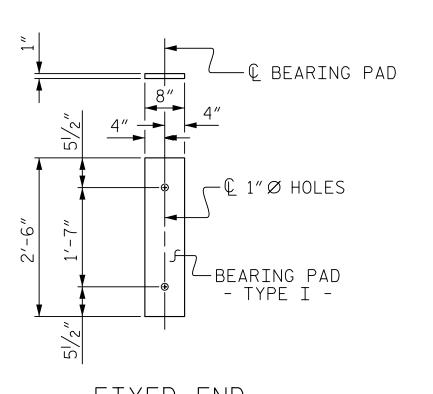
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

PRESTRESSED CONCRETE CORED SLAB UNIT

SHEET NO. REVISIONS S-4 NO. BY: BY: DATE: DATE: TOTAL SHEETS

STD. NO. 24PCS4_33_90S





FIXED END
(TYPE I - 22 REQ'D)

ELASTOMERIC BEARING DETAILS

1'-0"

10"

___#5 S13

(TYP.)

2³/₈"CL.

-#5 S12 SEE "PLAN OF

UNIT" FOR SPACING

<u>'2"CL.</u> | MIN.

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

GRADE 270 STRANDS

O.6" Ø L.R.

AREA
(SQUARE INCHES)

ULTIMATE STRENGTH
(LBS. PER STRAND)

APPLIED PRESTRESS
(LBS. PER STRAND)

43,950

CORED	RED SLABS REQUIRED								
	NUMBER	LENGTH	TOTAL LENGTH						
70'UNIT									
EXTERIOR C.S.	2	70′-0″	140′-0″						
INTERIOR C.S.	9	70′-0″	630′-0″						
TOTAL	11	70′-0″	770′-0″						

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
70'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	21/4″ ╽
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	3⁄4″ ♦
FINAL CAMBER	11/2"

** INCLUDES FUTURE WEARING SURFACE

Ĺ ¼″EXP.JT.MAT′L HELD IN

PLACE WITH GALVANIZED NAILS.

(NOTE: OMIT EXP. JT. MAT'L.

WHEN SLIP FORM IS USED)

21/2"

SECTION S-S

AT DAM IN OPEN JOINT

(THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

10"

1" FIELD BEND

"B" BARS

FIELD-CUT

#5 S13

END OF RAIL DETAILS

CHAMFER

CONST. JT.

CONST. JT.-

SIDE VIEW

2'-0"

\|FIELD CUT||

CHAMFER

#5 S12 & S13

ELEVATION AT EXPANSION JOINTS

6″CTS.

† † † †

BAR TYPES | S15 1'-8|/2" | S14 2'-7" | S11 2'-8" | S10 1'-9" | O15 | S10 1'-9" | O1

ALL BAR DIMENSIONS ARE OUT TO OUT

				EXTERI	OR UNIT	INTERI	OR UNI			
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIG			
B22	6	#4	STR	24'-6"	98	24'-6"	98			
S10	8	#5	3	4'-9"	40	4'-9"	40			
S11	144	#4	3	5′-10″	561	5′-10″	561			
* S12	79	#5	1	5′-7″	460					
S14	4	#4	3	5′-7″	15	5′-7″	15			
S15	4	#5	3	7'-1"	30	7′-1″	30			
DETNE	ODOTNO	^TEE!	1.00		7.4.4		7.4			
	ORCING :		LBS	.	744		744			
	* EPOXY COATED REINFORCING STEEL LBS. 460									
7000	P.S.I.CO	NCRETE	CU. YDS) ₀	11.8		11.8			

GUTTERLINE ASPH	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
70'UNITS	2"	3′-8″

0.6″∅ L.R. STRANDS

CONCRETE	RELEA	4SE	STRENGTH	
UNIT			PSI	
70'UNITS			5500	

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ % DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\sqrt{2}$ IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

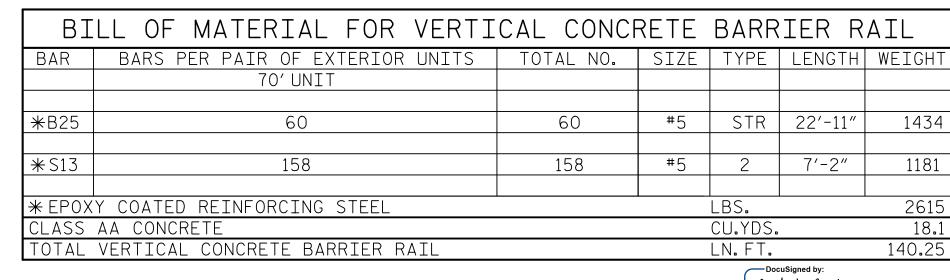
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

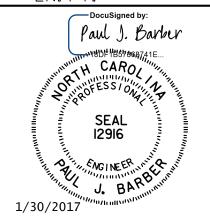
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.





PROJECT NO. 17BP.3.R.50

SAMPSON COUNTY

STATION: 18+23.00 -L-

SHEET 3 OF 3

DEPARTMENT OF TRANSPORTATION

STANDARD

3'-0" X 2'-0"

PRESTRESSED CONCRETE

CORED SLAB UNIT

DOCUMENT NOT CONSIDERED FINAL
UNLESS ALL SIGNATURES COMPLETED

ONLESS ALL SIGNATURES COMITE								
HNTB NORTH CAROLINA, P.C.				REVIS	SIONS	5		SHEET NO.
HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Ralei	gh, N.C. 27609	NO.	BY:	DATE:	NO.	BY:	DATE:	S-6
DRAWN BY J. BAYNE DATE 12/16		1			3			TOTAL SHEETS
CHECKED BY P. BARBER DATE 1/17	DWG. NO. 6	2			4			14

VERTICAL CONCRETE BARRIER RAIL DETAILS

FIELD CUT-#5 S13

#5 S12—

END VIEW

ASSEMBLED BY: J.BAYNE DATE: 12/16
CHECKED BY: P.BARBER DATE: 1/17

DRAWN BY: MAA 6/10
CHECKED BY: MKT 7/10 REV. 11/14 MAA/TMG

CONST. JT. —

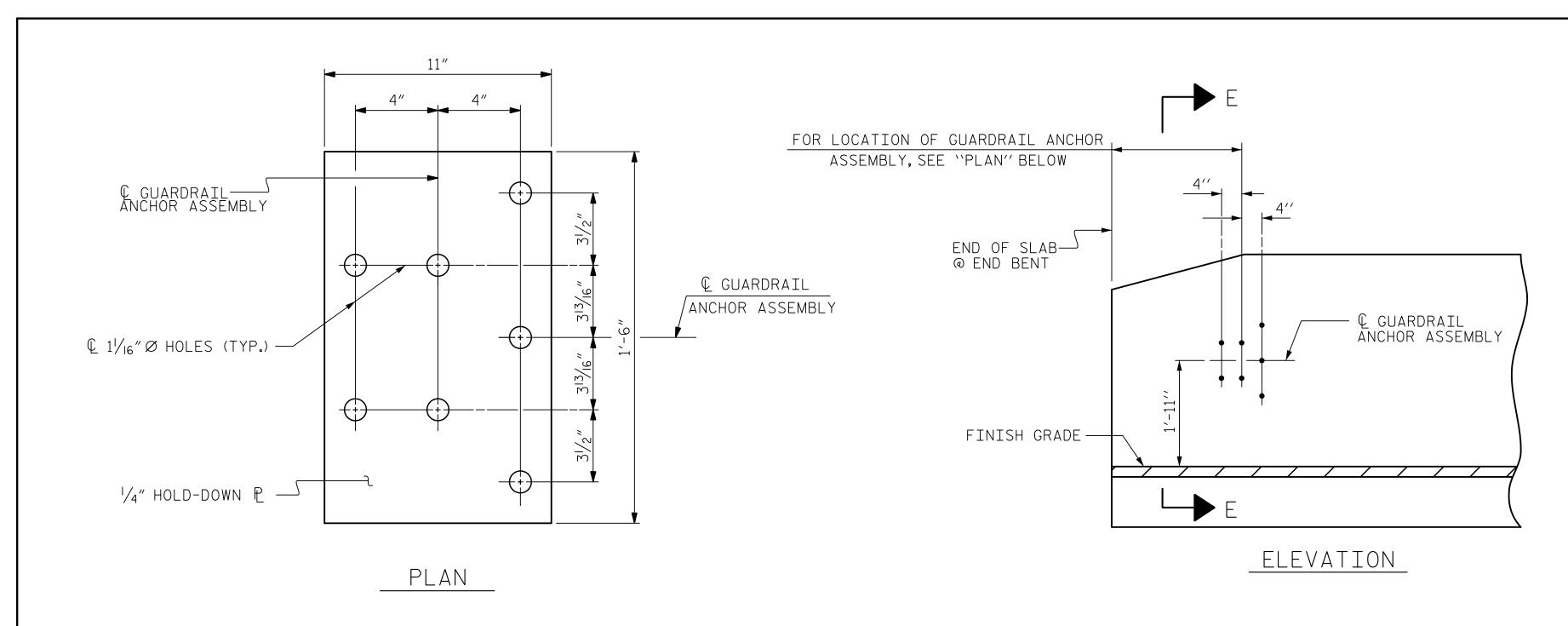
SECTION THRU RAIL

0

3'-9'/2" "GUTTERLINE ASPHA RAIL HEIGHT" TABI

> VARIES (SEE THICKNESS

> > STD. NO. 24PCS3_33_90S



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

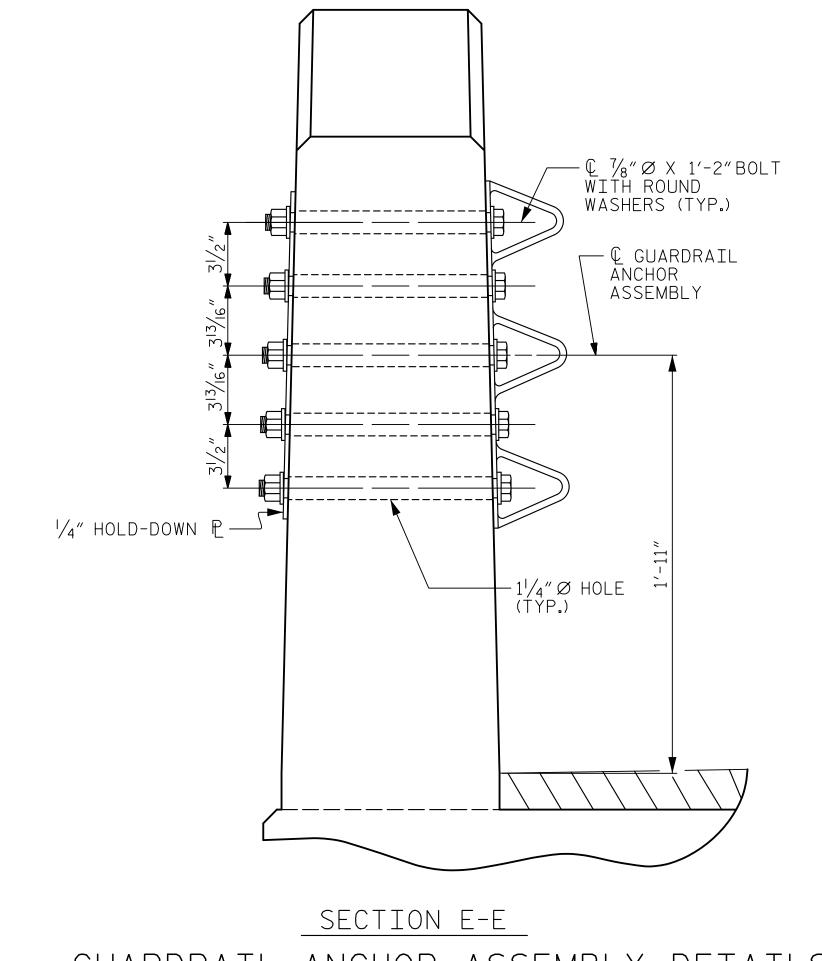
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

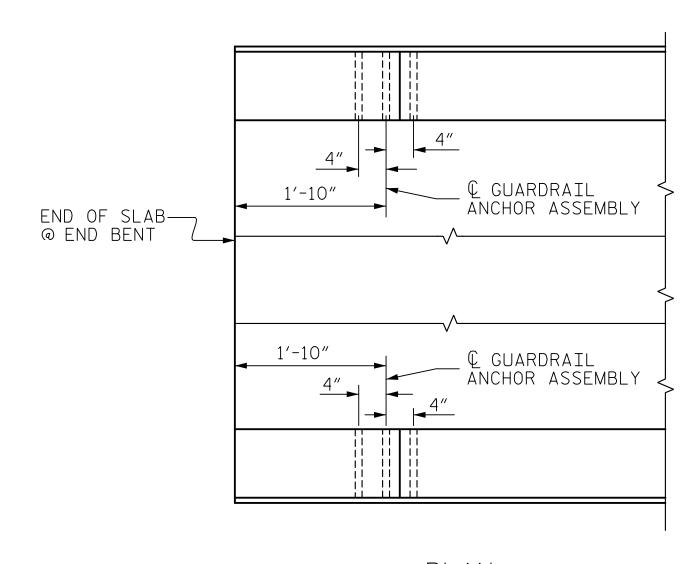
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " \varnothing HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



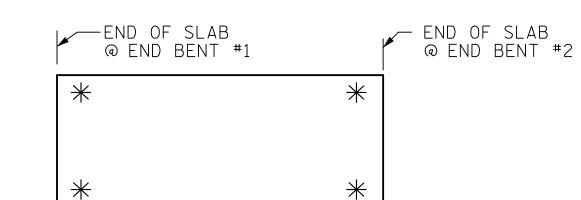
GUARDRAIL ANCHOR ASSEMBLY DETAILS



PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



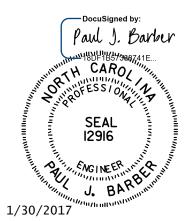
SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. 17BP.3.R.50 SAMPSON COUNTY

18+23.00 -L-STATION:

STATE OF NORTH CAROLINA



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 DRAWN BY J. BAYNE DATE 12/16
CHECKED BY P. BARBER DATE 1/17

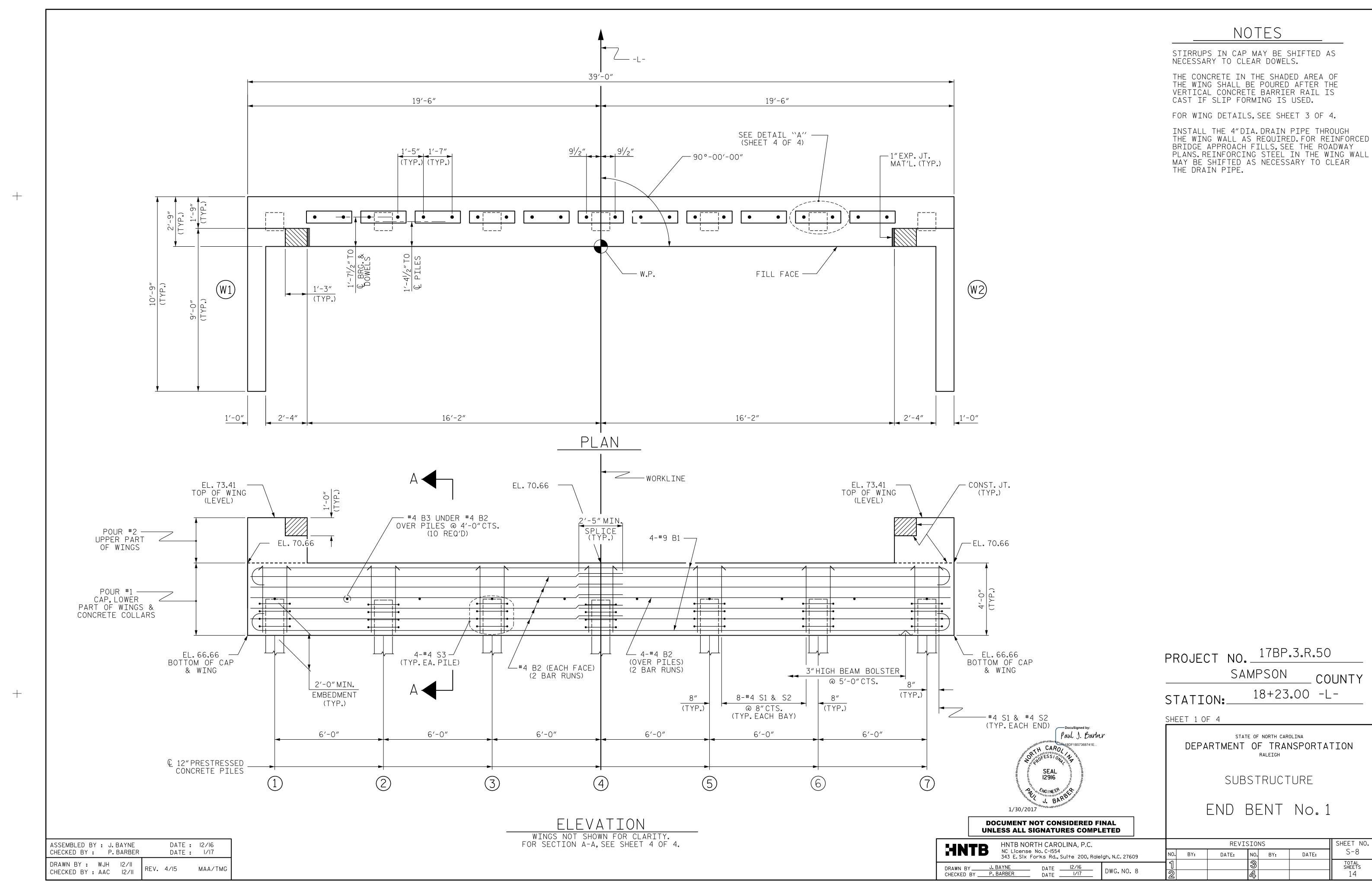
DEPARTMENT OF TRANSPORTATION STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL

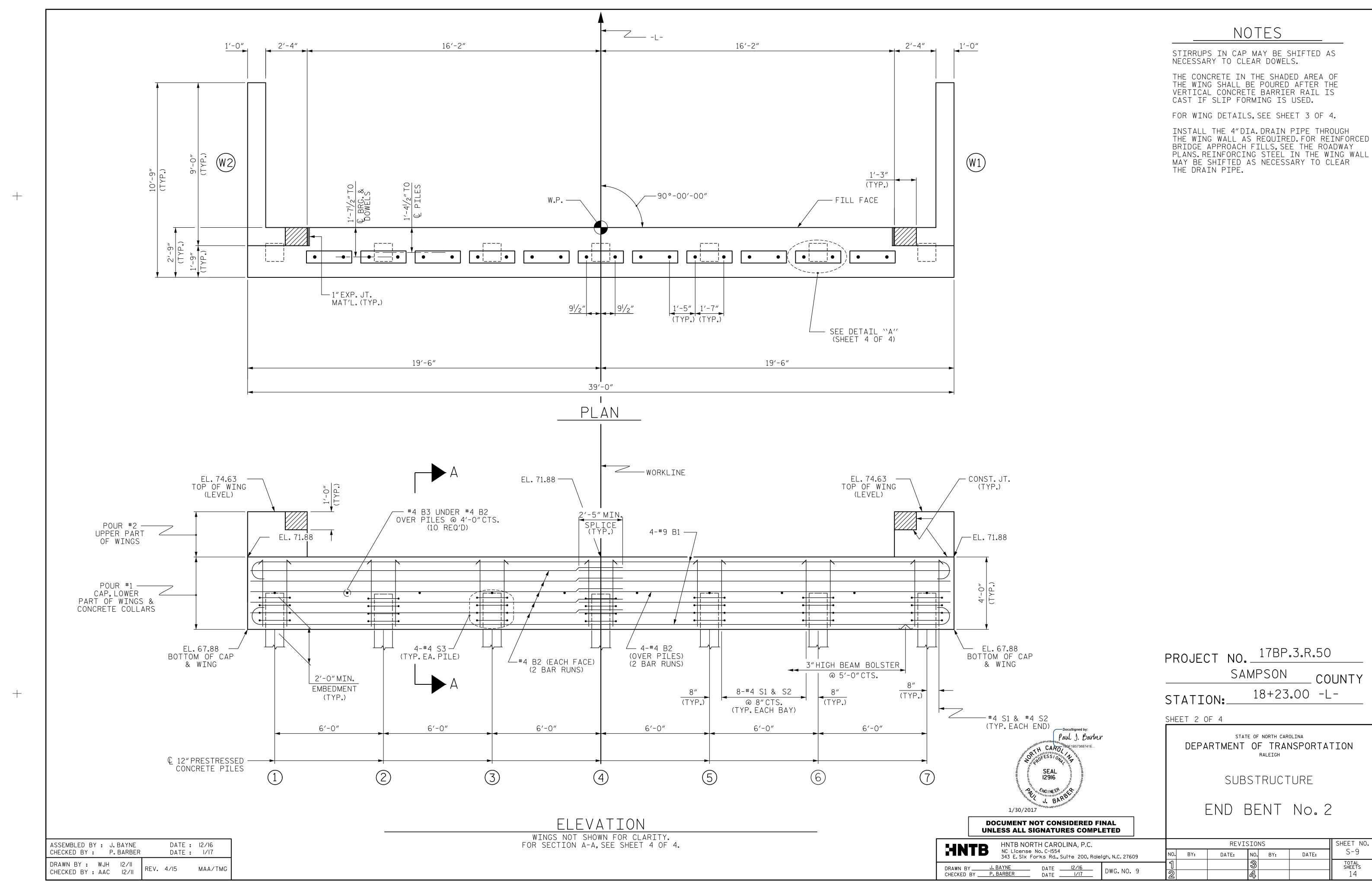
SHEET NO. REVISIONS S-7 NO. BY: DATE: BY: DATE: TOTAL SHEETS

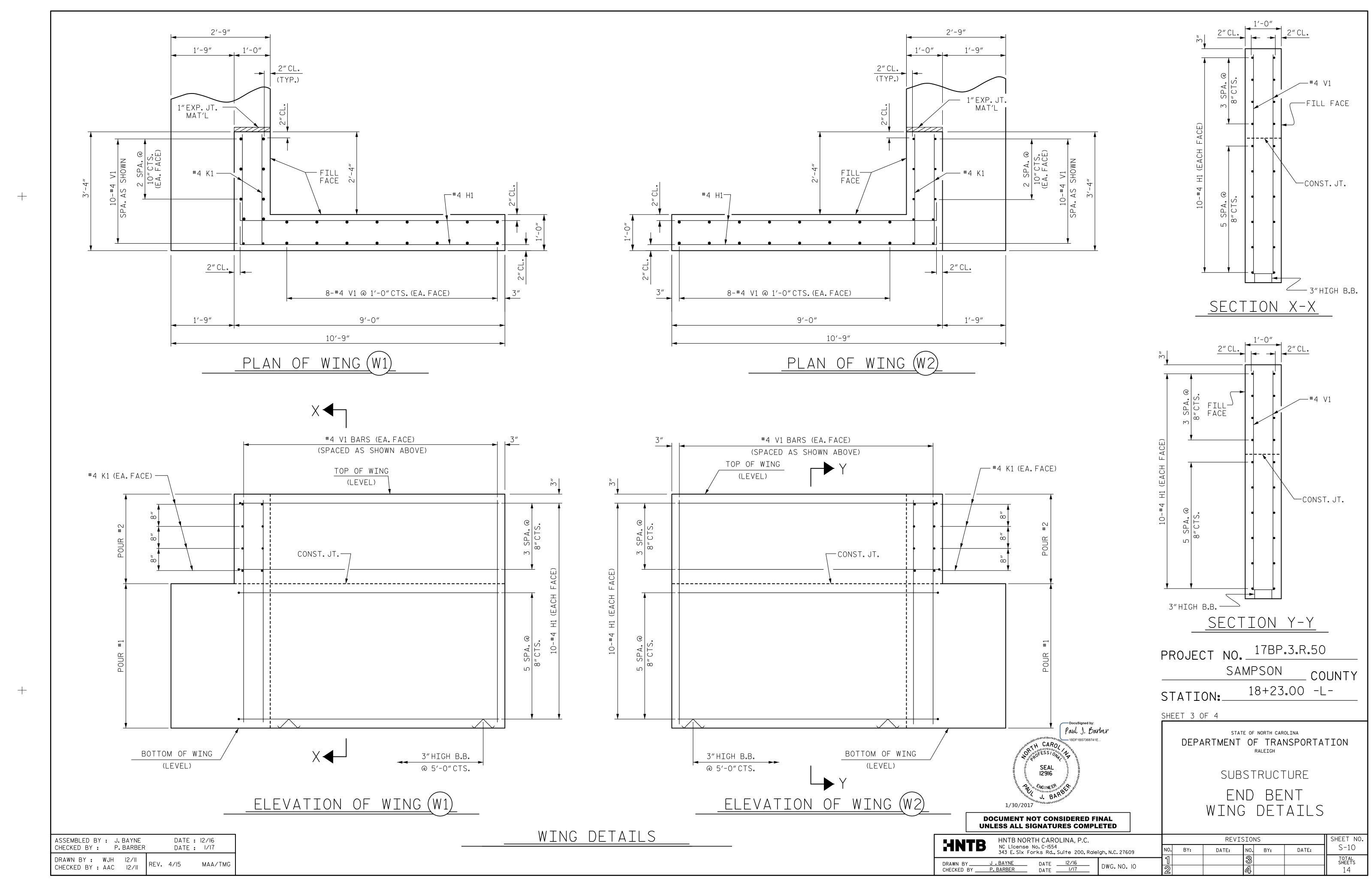
CHECKED BY: P. BARBER DATE: 1/17 DRAWN BY: MAA 5/10 MAA/GM CHECKED BY : GM 5/10 MAA/TMG

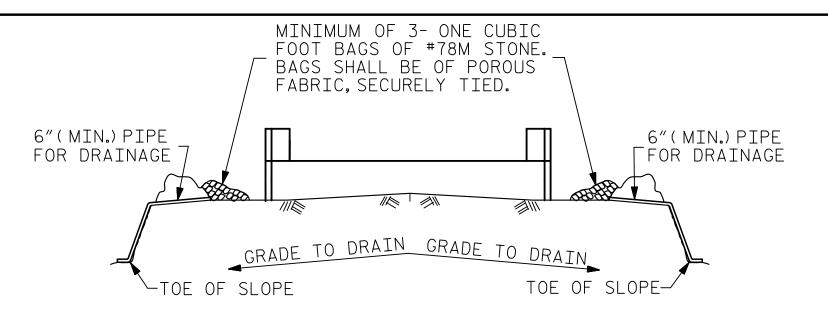
DATE: 12/16

ASSEMBLED BY : J. BAYNE







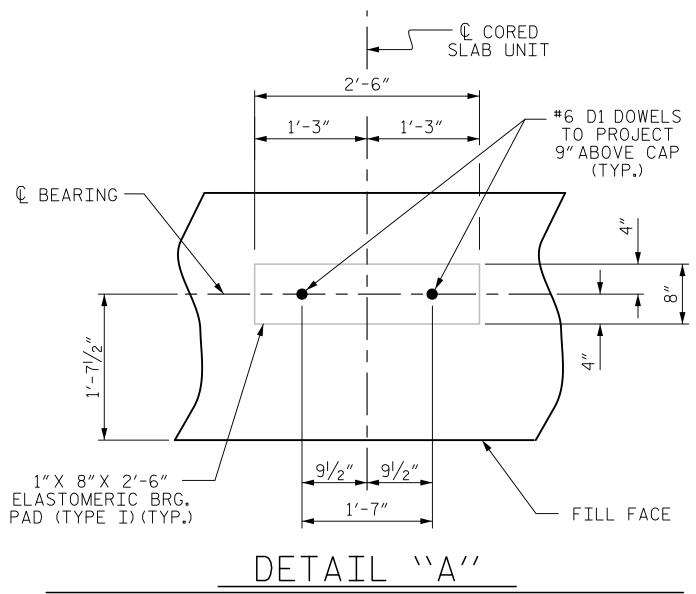


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

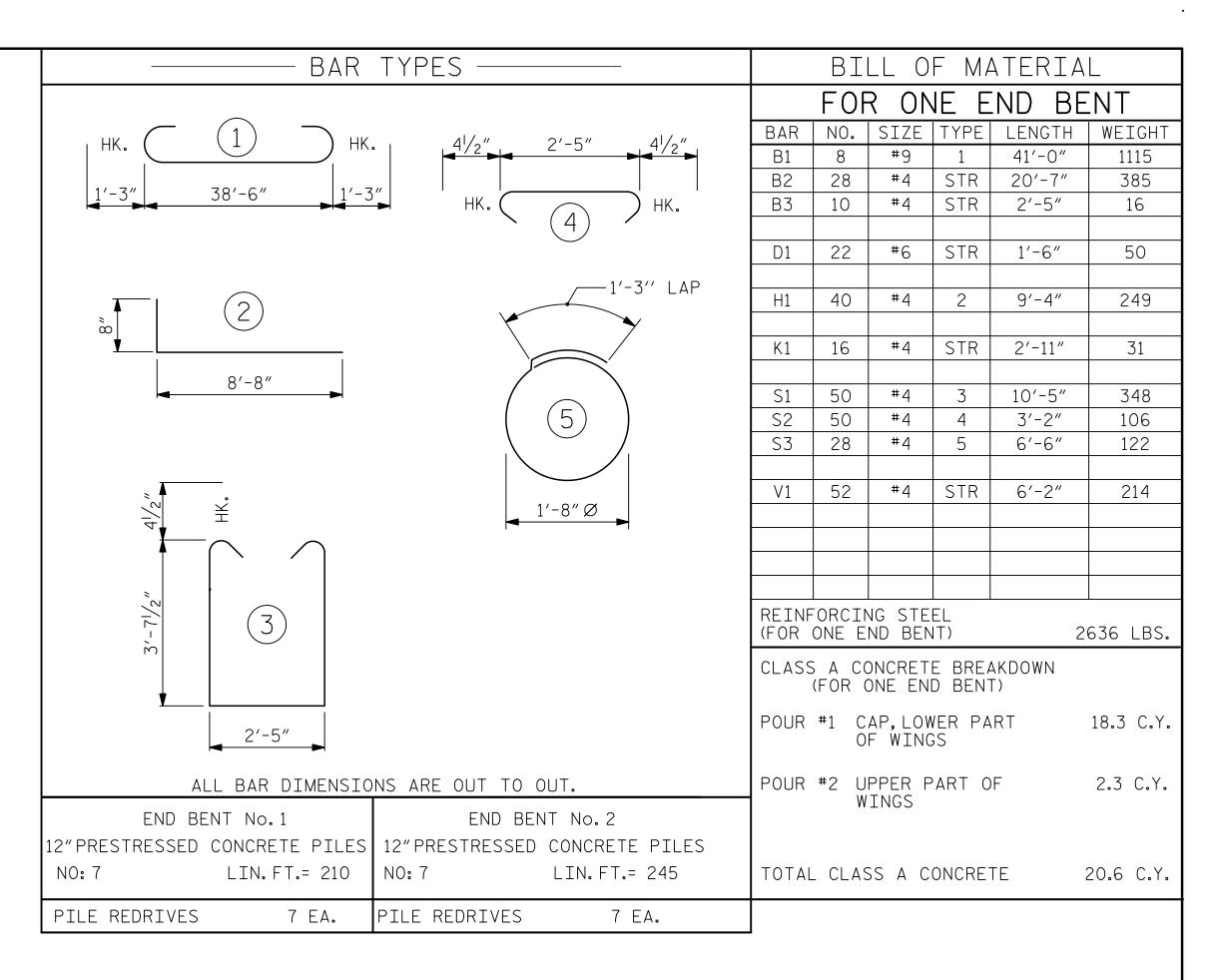
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

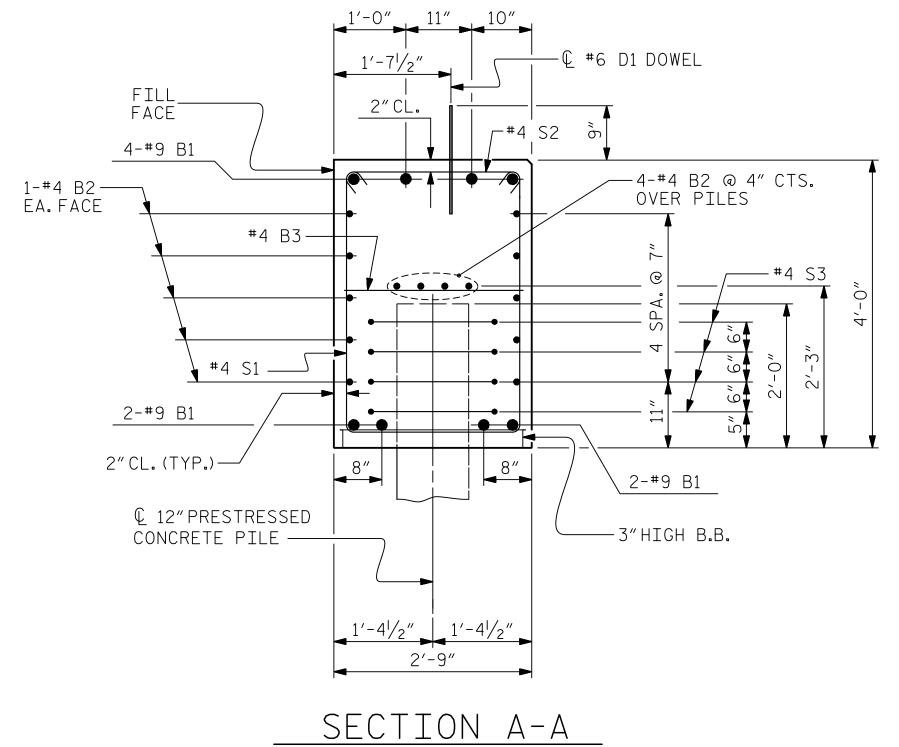
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)





PROJECT NO. 17BP.3.R.50

___SAMPSON___ COUNTY

STATION: 18+23.00 -L-

SHEET 4 OF 4

BY:

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

SHEET NO.

S-11

TOTAL SHEETS

DATE:

END BENT No.1 & 2 DETAILS

REVISIONS

DATE:

HNTB NORTH CAROLINA, P.C.

NC License No. C-1554
343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609

DRAWN BY J. BAYNE DATE 12/16
CHECKED BY P. BARBER DATE 1/17

DWG. NO. II

1/30/2017

Paul J. Barber

SEAL 12916

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ASSEMBLED BY: J. BAYNE
CHECKED BY: P. BARBER

DATE: 12/16
DATE: 1/17

DRAWN BY: WJH 12/11
CHECKED BY: AAC 12/11

STD. NO. EB_33_90S4

NO. BY:

ASSEMBLED BY: J. BAYNE

DRAWN BY: FCJ 7/88

CHECKED BY: CRK 3/89

CHECKED BY: P. BARBER

DATE : 1/17

DATE : 1/17

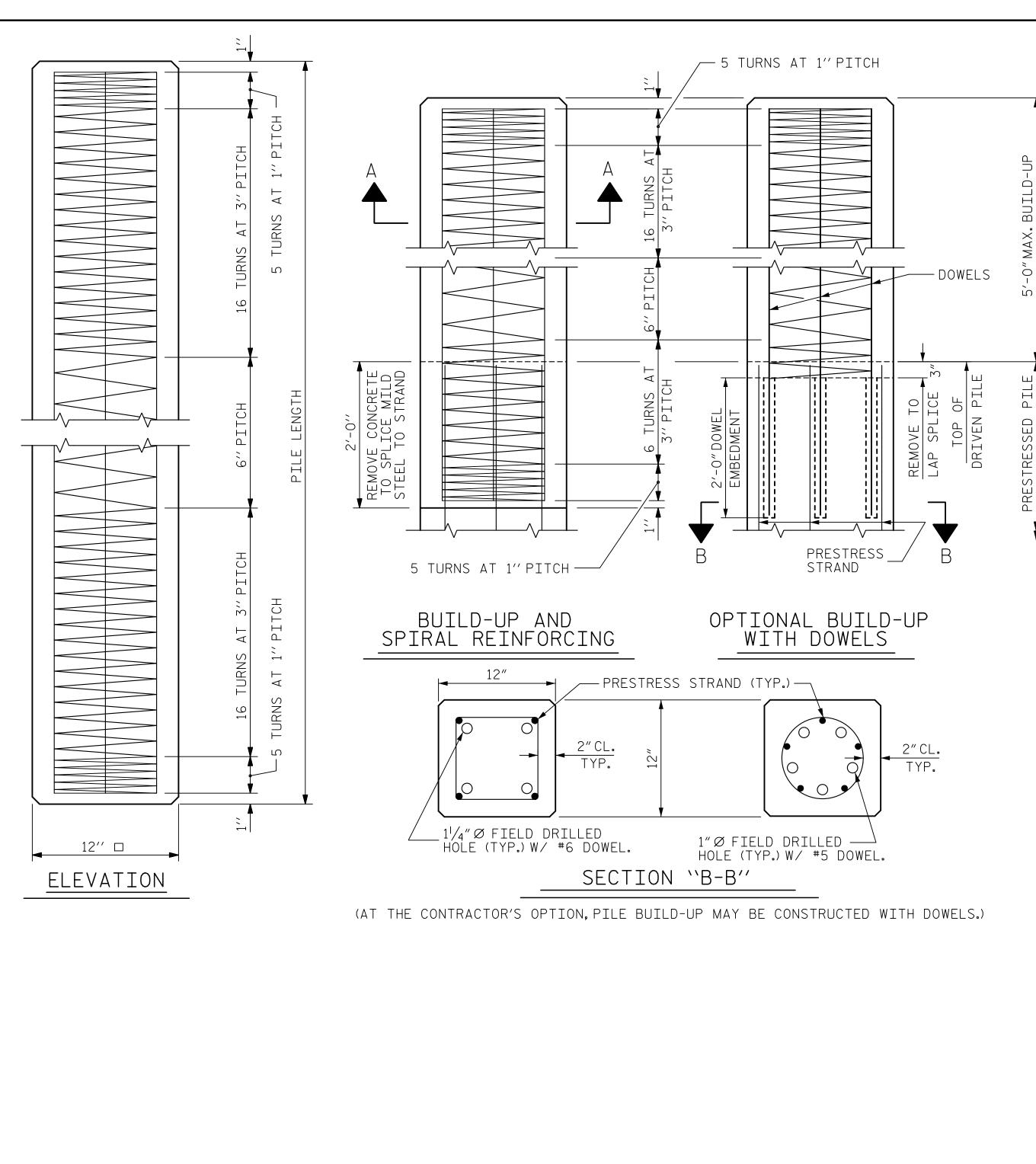
REV. 10/1/11

REV. 12/14

WMC/GM

MAA/GM

MAA/TMG



QUAI	NTITIES	FOR ON	NE 12" PI	RESTRES	SED PIL	Ε		
	CONCRETE	PILE WT.	ONE POIN	T PICK-UP	TWO POINT PICK-UP			
LENGTH	CU. YDS.	TONS	0.300L	0.700L	0.207L	0.586L		
25′-0′′	0.91	1.85	7′-6′′	17′-6′′				
30′-0′′	1.10	2.22	9'-0''	21'-0''				
35′-0′′	1.28	2.59	10'-6''	24'-6''				
40′-0′′	1.46	2.96	12'-0''	28'-0''				
45′-0′′	1.64	3.33	13′-6′′	31′-6′′				
50′-0′′	1.83	3.72	15′-0′′	35′-0′′				
55′-0′′	2.01	4.09			11'-4 /2''	32′-3′′		
60′-0′′	2.19	4.46			12′-5′′	35′-2′′		
65′-0′′	2.38	4.81			13'-51/2''	38′-1′′		
70′-0′′	2.57	5.18			14'-6''	41′-0′′		

5 PRESTRESS STRANDS

└─ W4.O COLD DRAWN STEEL WIRE SPIRAL─

 $^{\prime}$ - W4.0 cold drawn steel wire spiral

L THRU 50'

ONE POINT PICK-UP

TWO POINT PICK-UP

PICK-UP POINTS

4 PRESTRESS

STRANDS

2" CL.

TYP.

2" CL.

TYP.

SECTION "A-A"

SECTION "A-A"

 $\frac{1}{2}$ " or 0.6" \alpha Grade 270 L.R. Prestress strands

 $\frac{1}{2}$ " OR 0.6" Ø GRADE 270 L.R. PRESTRESS STRANDS

3/4"

TYP.

2" CL.

TYP.

TYP.

TYPICAL SECTION

TYPICAL SECTION

0.700L

5-#5 BARS

-4- #5 BARS

TYPICAL PATTERN

TYPICAL PATTERN

FOR BURNING STRANDS

FOR BURNING STRANDS

NOTES

PRESTRESSED CONCRETE STRENGTH: f'c = 7,500 PSI BUILD-UP CONCRETE STRENGTH: f'c = 7,500 PSI

STRAND DATA:

SIZE	GRADE	AREA	ULTIMATE STRENGTH	APPLIED PRESTRESS FORCE
1/2′′	270 L.R.	0.153	41,300# PER STRAND	30,980# PER STRAND
0.6"	270 L.R.	0.217	58,600# PER STRAND	43,940# PER STRAND

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS CONFORMING TO AASHTO M203.STRAND SAMPLING REQUIREMENTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

AT THE CONTRACTOR'S OPTION, $\frac{1}{2}$ " OR 0.6" STRANDS MAY BE USED IN EITHER THE 4 OR 5 STRAND CONFIGURATION SHOWN IN THE TYPICAL SECTION DETAIL. MIXING OF STRAND SIZE IS NOT ALLOWED.

THE SLIP-FORM METHOD OF CASTING PILES WILL NOT BE PERMITTED.

TRANSFER THE LOAD FROM THE ANCHORAGES TO THE PILE AFTER THE CONCRETE HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 4.000 PSI.

IF STRAND STRESS IS RELIEVED BY BURNING, THE STRANDS SHALL BE BURNED IN PAIRS, EXCEPT WHERE 5 STRANDS ARE USED, THE LAST STRAND MAY BE BURNED SINGLY ACCORDING TO BURNING PATTERNS SHOWN. NOT MORE THAN 4 STRANDS MAY BE BURNED AT ANY ONE SECTION BEFORE THE SAME STRANDS ARE BURNED AT BOTH ENDS OF THE BED AND BETWEEN EACH PAIR OF PILES IN THE BED.

PROPOSED DEVICES FOR LIFTING PILES, RECESS DETAILS, AND PATCHING MATERIAL SHALL BE DETAILED IN SHOP DRAWINGS. AFTER ATTACHMENTS HAVE BEEN REMOVED, OPENINGS SHALL BE REPAIRED SUCH THAT THE APPEARANCE OF THE PILE IS UNIFORM.

WHERE CAST-IN-PLACE LIFTING DEVICES ARE NOT USED, PICK-UP POINTS ARE TO BE INDICATED WITH A 2" WIDE BLACK MARK.

DRIVE PILES USING A METHOD APPROVED BY THE ENGINEER, WHEREBY THE HEAD OF THE PILE IS NOT DAMAGED.

DRIVING OF THE BUILT-UP PILE WILL NOT BE PERMITTED UNTIL THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF 5,000 PSI AND UNTIL A PERIOD OF SEVEN DAYS HAS ELAPSED SINCE CASTING OF THE BUILD-UP.

DOWEL INSTALLATION FOR OPTIONAL BUILD-UP

GROUT COMPRESSIVE STRENGTH: f'c= 5.000 PSI

BEFORE DRILLING DOWEL HOLES, REMOVE THE UPPER 3"OF CONCRETE FROM THE TOP OF THE PILE WITHOUT DAMAGE TO THE REINFORCING STEEL. THE REMOVAL PLANE SHOULD BE NORMAL TO THE EDGE OF THE PILE.

DOWEL HOLES SHALL BE POSITIONED TO MAINTAIN $\frac{1}{2}$ CLEAR TO ALL EXISTING PRESTRESSING STRANDS IN THE CONCRETE PILE.

FIELD DRILLED HOLES SHALL BE CLEAN AND FREE OF ANY OBSTRUCTIONS BEFORE GROUTING OF DOWELS. DOWEL BARS SHALL BE INSTALLED AND GROUTED WITH AN APPROVED NON-SHRINK GROUT.

THE SPIRAL REINFORCING IN ALL BUILD-UPS SHALL BE W4.0 COLD DRAWN WIRE WHICH SHALL BE SECURED TO THE LONGITUDINAL REINFORCEMENT TO MAINTAIN PITCH.

THE SPIRAL REINFORCING IN THE BUILD-UP AND THE PRESTRESSED CONCRETE PILE SHALL BE SPLICED BY OVERLAPPING A MIN. OF ONE TURN.

PROJECT NO. 17BP.3.R.50

SAMPSON

COUNTY

STATION: 18+23.00 -L-

DocuSigned by:

Paul J. Barbur

Paul J. Barbur

CARO

DRAWN BY J. BAYNE DATE 1/17
CHECKED BY P. BARBER DATE 1/17

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HNTB NORTH CAROLINA, P.C.

NC License No. C-1554

343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609

DWG. NO. 12

12" PRESTRESSED CONCRETE PILE

STANDARD

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

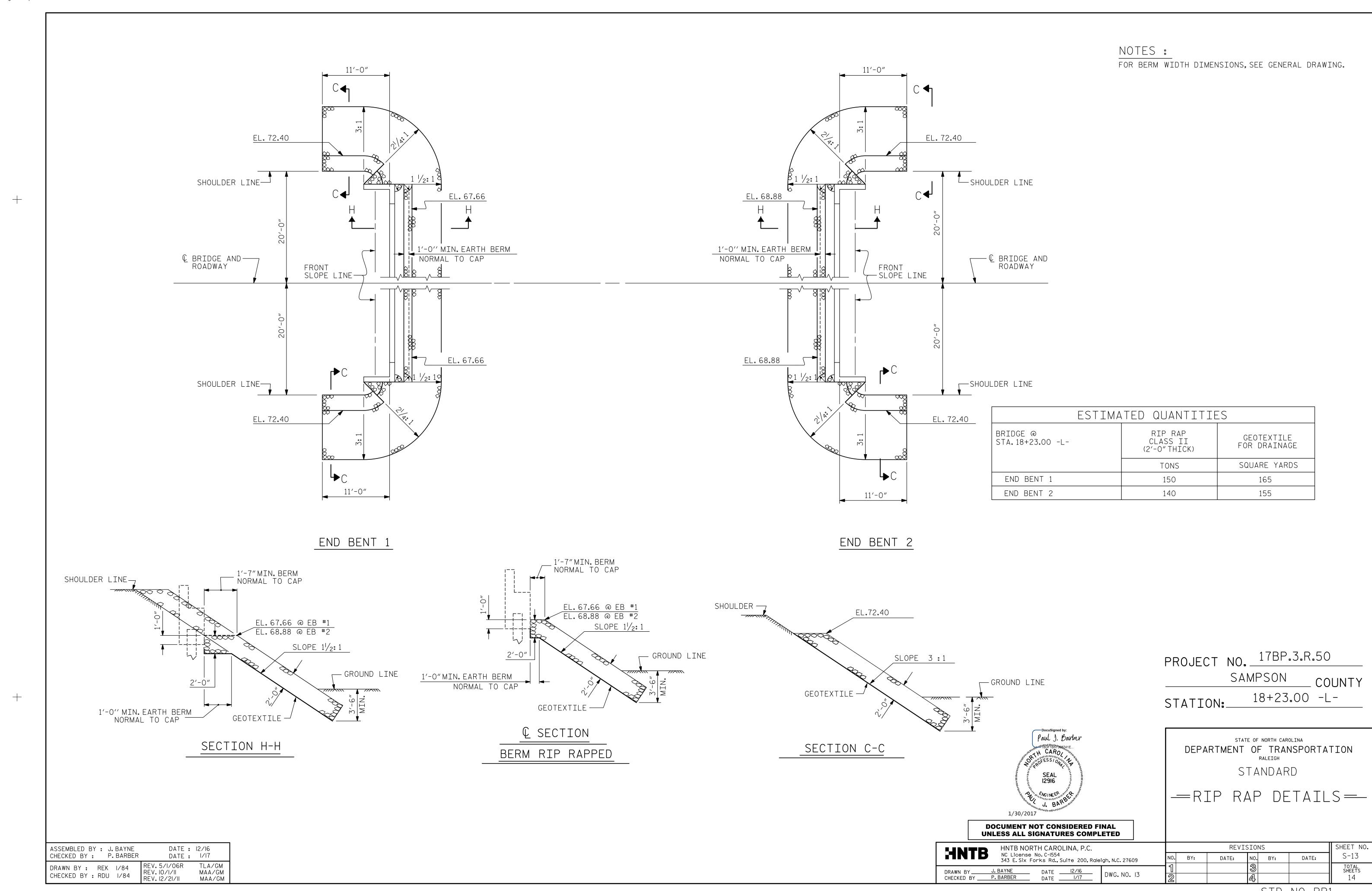
RALEIGH

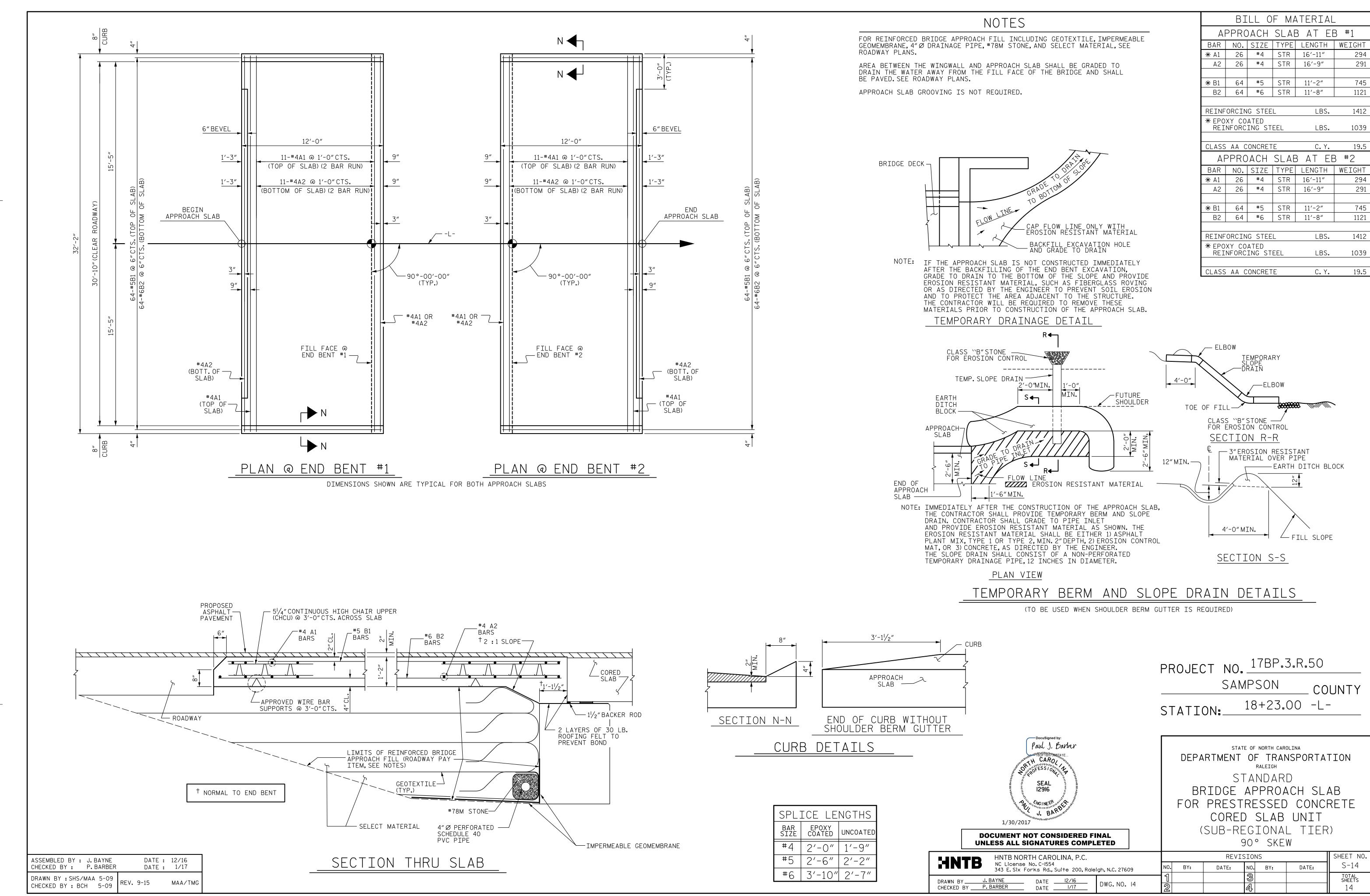
REVISIONS

NO. BY: DATE: NO. BY: DATE: S-12

1 3 TOTAL SHEETS
14

STD. NO. PCP1





STANDARD NOTES

DESIGN DATA:

A.A.S.H.T.O. (CURRENT) SPECIFICATIONS - - - - - - - - - - - -LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O.

STRESS IN EXTREME FIBER OF

STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS.PER SQ.IN.

REINFORCING STEEL IN TENSION

GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ----- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O.

STRUCTURAL TIMBER - TREATED OR

UNTREATED - EXTREME FIBER STRESS ---- 1.800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN

375 LBS. PER SQ. IN. OF TIMBER ----

EQUIVALENT FLUID PRESSURE OF EARTH - - - - -

30 LBS. PER CU. FT.

(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION. VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN, AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $3\!\!\!/_4$ "ø studs specified on the plans. This substitution shall be made at THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH